GOVERNING CODE 2021 INTERNATIONAL BUILDING CODE - NEW JERSEY EDITION I. DEAD LOADS A. R*00*F B. CEILING/ATTIC C. TYPICAL FLOOR = 15 PSF + STEEL BEAM SELF WT LIVE LOADS A. ROOF = 25 PSF (MINIMUM) B. CEILING/ATTIC = 10 PSF (C. 2ND & 3RD FLOOR OFFICE = 50 PSF D. 2ND & 3RD FLOOR CORRIDORS = 80 PSF = 10 PSF (NO STORAGE) . IST FLOOR = 100 PSF SNOW LOADS A. GROUND SNOW LOAD = 30.0 PSF B. FLAT - ROOF SNOW LOAD = 21.0 PSF SNOW EXPOSURE FACTOR CE = 1.0 THERMAL FACTOR CT = 1.0 (MAIN BUILDING AND VESTIBULES); 1.2 (UNHEATED CANOPIES) SNOW LOAD IMPORTANCE FACTOR, IS = 1.0 . ALL APPLICABLE EFFECTS DUE TO SNOW DRIFTING 4. WIND LOADS A. BASIC WIND SPEED - 115 mph (ULTIMATE) B. WIND LOAD IMPORTANCE FACTOR = 1.0 . RISK CATEGORY = II D. WIND EXPOSURE CATEGORY B FOR MAIN WIND FORCE RESISTING SYSTEM E. WIND EXPOSURE CATEGORY B FOR COMPONENTS AND CLADDING E. WIND DESIGN PRESSURES - MWFRS GENERAL NOTES WORK SHALL BE PERFORMED IN ACCORDANCE WITH THE "2021 INTERNATIONAL BUILDING CODE - N.J. EDITION" AND ALL FEDERAL, STATE AND CITY LAWS, BYLAWS, ORDINANCES AND REGULATIONS IN ANY MANNER AFFECTING THE CONDUCT OF THIS WORK AS WELL AS ALL ORDERS OR DECREES WHICH HAVE BEEN PROMULGATED OR ENACTED BY ANY LEGAL BODIES OR TRIBUNALS HAVING AUTHORITY OR JURISDICTION OVER THE WORK, MATERIALS, EMPLOYEES OR CONTRACT. THE CONTRACTOR SHALL BE RESPONSIBLE FOR MAINTAINING PERSONNEL SAFETY ON THE JOBSITE. GUIDELINES FOR CONSTRUCTION SAFETY SHALL BE IN ACCORDANCE WITH, BUT NOT LIMITED TO, THE CONSTRUCTION INDUSTRY OSHA SAFETY AND HEALTH STANDARDS (1926 STANDARDS), AND ANY LOCAL ORDINANCES OR CODES WHICH 2. IN CASE OF CONFLICT BETWEEN THE GENERAL NOTES, SPECIFICATIONS AND DETAILS, THE MOST RIGID REQUIREMENTS SHALL GOVERN. 3. SEE ARCHITECTURAL DRAWINGS FOR LOCATIONS OF MASONRY AND DRYWALL NON-LOAD BEARING PARTITIONS, PROVIDE SLIP CONNECTIONS THAT ALLOM VERTICAL MOVEMENT THE HEADS OF ALL SUCH PARTITIONS. CONNECTIONS SHALL BE DESIGNED TO SUPPORT THE TOP OF THE WALLS LATERALLY FOR THE CODE-REQUIRED LOAD. 4. ALL COSTS OF INVESTIGATION AND/OR REDESIGN DUE TO THE CONTRACTOR IMPROPER INSTALLATION OF STRUCTURAL ELEMENTS OR OTHER ITEMS NOT IN CONFORMANCE WITH THE CONTRACT DOCUMENTS SHALL BE AT THE CONTRACTORS EXPENSE 5. THE STRUCTURAL DRAWINGS SHALL BE USED IN CONJUNCTION WITH THE SPECIFICATIONS, ARCHITECTURAL AND MECHANICAL DRAWINGS. IF HERE IS A DISCREPANCY BETWEEN DRAWINGS, IT IS THE CONTRACTORS RESPONSIBILITY TO NOTIFY THE ARCHITECT PRIOR TO PERFORMING THE WORK. 6. THE CONTRACTOR SHALL VERIFY ALL EXISTING BUILDING INFORMATION SHOWN (DIMENSIONS, ELEVATIONS, ETC.) AND NOTIFY THE ARCHITECT/ENGINEER OF ANY DISCREPANCIES PRIOR TO FABRICATION OF ANY STRUCTURAL COMPONENT. FAILURE TO NOTIFY ARCHITECT/ENGINEER OF UNSATISFACTORY CONDITIONS CONSTITUTES ACCEPTANCE OF UNSATISFACTORY CONDITIONS. I. IF THE EXISTING FIELD CONDITIONS DO NOT PERMIT THE INSTALLATION OF THE WORK IN ACCORDANCE WITH THE DETAILS SHOWN, THE CONTRACTOR SHALL NOTIFY THE ARCHITECT/ENGINEER IMMEDIATELY AND PROVIDE A SKETCH OF THE CONDITION WITH HIS PROPOSED MODIFICATION OF THE DETAILS GIVEN ON THE CONTRACT DOCUMENTS. DO NOT COMMENCE WORK UNTIL CONDITION IS RESOLVED AND MODIFICATION IS APPROVED BY THE ARCHITECT. 8. THE CONTRACTOR SHALL SUBMIT, FOR REVIEW, DRAWINGS AND CALCULATIONS FOR ALL PERFORMANCE ASSEMBLIES IDENTIFIED IN THE GENERAL NOTES AND LISTED BELOW: THE DESIGN OF THESE ASSEMBLIES IS THE RESPONSIBILITY OF THE CONTRACTOR'S ENGINEER REGISTERED IN THE PROJECT'S JURISDICTION. ALL SUBMITTALS SHALL BEAR THIS ENGINEER'S SEAL AND SIGNATURE. REVIEW SHALL BE FOR GENERAL CONFORMANCE WITH THE PROJECT REQUIREMENTS AS INDICATED ON THE DRAWINGS AND IN THE GENERAL NOTES 8.I. METAL STAIRS AND METAL RAILINGS; DESIGNS SHALL TAKE INTO ACCOUNT ALL VERTICAL AND LATERAL LOADS REQUIRED BY APPLICABLE BUILDING CODES, WHERE HEADERS OR OTHER TYPES OF STRUCTURAL MEMBERS HAVE BEEN DESIGNATED BY THE STRUCTURAL ENGINEER OF RECORD TO SUPPORT THE STAIRS, THE CONNECTIONS FROM THE STAIRS SHALL BE DESIGNED SO THAT NO ECCENTRIC OR TORSIONAL FORCES ARE INDUCED IN THESE STRUCTURAL MEMBERS. THE CONTRACTOR SHALL BE RESPONSIBLE FOR FURNISHING AND INSTALLING HARDWARE AS REQUIRED BY THE STAIR DESIGN. 9. SHOP DRAWINGS FOR ALL STRUCTURAL MATERIALS TO BE SUBMITTED TO ARCHITECT FOR REVIEW PRIOR TO THE START OF FABRICATION OR COMMENCEMENT OF WORK, REVIEW PERIOD SHALL BE A MINIMUM OF TWO (2) WEEKS, REPRODUCTION OF ANY PORTION OF THE STRUCTURAL CONTRACT DRAWINGS FOR RESUBMITTAL AS SHOP DRAWINGS IS PROHIBITED. SHOP DRAWINGS PRODUCED IN SUCH A MANNER WILL BE REJECTED AND RETURNED IO. SHOP DRAWINGS SHALL BEAR THE CONTRACTOR'S STAMP OF APPROVAL WHICH SHALL CONSTITUTE CERTIFICATION THAT THE CONTRACTOR HAS VERIFIED ALL CONSTRUCTION CRITERIA, MATERIALS, AND SIMILAR DATA AND HAS CHECKED EACH DRAWING FOR COMPLETENESS, COORDINATION, AND COMPLIANCE WITH THE CONTRACT DOCUMENTS . THE CONTRACTOR SHALL COORDINATE PRINCIPAL OPENINGS IN THE STRUCTURE AS INDICATED ON THE CONTRACT DOCUMENTS. REFER TO THE ARCHITECTURAL, MECHANICAL, ELECTRICAL, AND PLUMBING DRAWINGS FOR SLEEVES, CURBS, INSETS, ETC. NOT INDICATED. THE LOCATION OF SLEEVES OR OPENINGS IN STRUCTURAL MEMBERS SHALL BE APPROVED BY THE STRUCTURAL ENGINEER.

IZ THE INSPECTION AND TESTING OF ALL SUBGRADE AND COMPACTED FARTHWORK SHALL BE CONDUCTED UNDER THE SUPERVISION OF A

QUALIFIED GEOTECHNICAL CONSULTANT. CONTRACTOR SHALL NOTIFY THE ARCHITECT OR STRUCTURAL ENGINEER 24 HOURS PRIOR TO PLACEMENT OF CONCRETE IN THE FOOTINGS. IF UNSUITABLE SUBGRADE SOILS ARE ENCOUNTERED, THE CONTRACTOR SHALL SUBMIT RECOMMENDATIONS PREPARED BY A GEOTECHNICAL CONSULTANT TO THE STRUCTURAL ENGINEER FOR APPROVAL.

13. THE CONTRACTOR SHALL PROVIDE BRACING AS REQUIRED TO MAINTAIN PLUMBNESS AND STABILITY DURING CONSTRUCTION. CONTRACTOR

15. THE SUBGRADE AND EACH LAYER OF FILL OR BACKFILL SHALL BE COMPACTED TO A DRY DENSITY AT LEAST EQUAL TO 45% OF THE

OF THE CONTRACTOR. THE CONTRACTOR SHALL TAKE ALL NECESSARY PRECAUTION TO MAINTAIN AND INSURE INTEGRITY OF THE STRUCTURE AT ALL STAGES OF CONSTRUCTION AND COORDINATION OF WORK WITH MECHANICAL AND ELECTRICAL WORK.

16. METHODS, PROCEDURES AND THE SEQUENCES (OTHER THAN THAT NOTED ON THE DRAWINGS) OF CONSTRUCTION ARE THE RESPONSIBILITY

WORK NOT INDICATED ON A PART OF THE DRAWINGS BUT REASONABLY IMPLIED TO BE SIMILAR TO THAT SHOWN AT CORRESPONDING

16. MINOR DETAILS OR INCIDENTAL ITEMS NOT SHOWN OR SPECIFIED, BUT NECESSARY FOR A PROPER AND COMPLETE INSTALLATION SHALL

19. MISCELLANEOUS WOOD OR COLD FORMED STEEL BLOCKING, FRAMING MEMBERS, ANCHORS, FASTENERS, ETC.. SHALL BE PROVIDED AS

SHALL PROVIDE SHORING TO MAINTAIN THE STRUCTURAL INTEGRITY OF THE EXISTING STRUCTURE

MAXIMUM DRY DENSITY ATTAINED BY THE MODIFIED PROCTOR TEST ASTM DI557.

PLACES SHALL BE PROVIDED BY THE CONTRACTOR AT NO ADDITIONAL COST.

REQUIRED WHETHER OR NOT SPECIFICALLY INDICATED ON DRAWINGS.

BE INCLUDED AS REQUIRED

14. THE SLAB-ON-GRADE SHALL BE UNDERLAIN BY A MINIMUM OF SIX INCHES OF STABLE GRANULAR MATERIAL.

Fb = 2,900 PSI FOR 12" DEPTH FOR OTHER MULTIPLY BY [12/d]0.111 Fv = 290 PSI Fc^ = 650 PSI E = 2,000,000 PSI 9. LVL (LAMINATED VENEER LUMBER) SHALL BE OF WIDTH AND DEPTH AS SPECIFIED ON DRAWINGS, MULTIPLE PLY MEMBERS SHALL BE ASSEMBLED IN ACCORDANCE WITH THE MANUFACTURERS ASSEMBLY DETAILS OR AS NOTED ON THE DRAWINGS. THE FOLLOWING MINIMUM STRUCTURAL PROPERTIES SHALL APPLY: Fb = 2,600 PSI FOR I2" DEPTH FOR OTHER MULTIPLY BY [12/d]0.136 Fv = 285 PSI Fc^ = 750 PSI E = 1,900,000 PSI

O. ALL STRUCTURAL FLOOR FRAMING SHALL BE DOUG-FIR NO.2 OR BETTER. THE FOLLOWING MINIMUM STRUCTURAL PROPERTIES SHALL Fb = 900 PSI Fc^ = 625 PSI (PERPENDICULAR TO GRAIN)

FOUNDATIONS

TOTAL LOAD.....2000 PSF (NET)

NCLUDING COMPACTION REQUIREMENTS

MOOD CONSTRUCTION

CONSTRUCTION" 2018 (WITH 2018 NDS SUPPLEMENT

EXCESSIVELY HIGH OR EXCESSIVELY LOW HUMIDITY

WCLIB - WEST COAST LUMBER INSPECTION BUREAU

WWPA - WESTERN WOOD PRODUCTS ASSOCIATION

For PERPENDICULAR = 625, Fv = 180, and E = 1,600,000.

SPIB - SOUTHERN PINE INSPECTION BUREAU

ARE CAPABLE OF PROVIDING THIS SUPPORT.

3. COMPLY WITH GRADING RULES OF GRADING AGENCY FOR SPECIES OF TIMBER USED

"S-DRY" OR SIMILAR INDICATION IN GRADE MARKING OR CERTIFICATION OF GRADE.

E = 1,600,000 PSI BASED DESIGN VALUES FOR WOOD STUD AND BRACING SHALL BE DOUG-FIR OR DOUG FIR STUD GRADE OR BETTER. THE FOLLOWING MINIMUM STRUCTURAL PROPERTIES SHALL APPLY: Fv = 180 PSI Ft. = 400 PSI

FOUNDATIONS HAVE BEEN DESIGNED AND FOOTING ELEVATIONS ESTABLISHED ON THE BASIS IN OF LOCAL MINIMUM CODE REQUIREMENTS

2. THE FOUNDATION FOR THE STRUCTURE HAS BEEN DESIGNED FOR THE FOLLOWING ALLOWABLE SOIL BEARING PRESSURES AT A BEARING DEPTH OF APPROXIMATELY 36" BELOW FINISHED FLOOR.

THE FOOTING LEVEL SHALL BE TESTED USING DROP-BAR PERCUSSION TEST OR PENETROMETER AT BEARING LEVELS TO ENSURE

3. THE BOTTOM OF EXTERIOR FOOTINGS SHALL BE A MINIMUM OF THREE (3) FEET BELOW FINISHED GRADE, OR AS REQUIRED BY LOCAL

5. SEE THE GEOTECHNICAL REPORT FOR EXCAVATION, BACKFILL AND PREPARATION OF THE FOUNDATION AND SLAB-ON-GRADE SUBGRADE

DENSIFIED SUBSTRATA PRIOR TO PLACING FILL MATERIAL EVENLY IN 8" THICK (MAXIMUM) LAYERS AND COMPACTING TO REQUIRED

THAT THE SAFE BEARING CAPACITY MEETS OR EXCEEDS THE DESIGN VALUE INDICATED ABOVE. REPORTS SHALL BE SUBMITTED TO THE

FOUNDATION EXCAVATIONS REMAIN DRY DURING CONSTRUCTION. ANY SHEETING OR SHORING REQUIRED FOR DEMATERING SHALL BE THE

8. IF CONDITIONS PROVE TO BE UNACCEPTABLE AT THE BEARING ELEVATIONS SHOWN, THE FOOTING BEARING ELEVATIONS MAY NEED TO BE LOWERED BASED ON THE RECOMMENDATIONS OF THE GEOTECHNICAL ENGINEER. FINAL BEARING ELEVATIONS AND BACKFILL RECOMMENDATIONS MUST BE APPROVED BY THE STRUCTURAL ENGINEER PRIOR TO FIELD MODIFICATION. CONCRETE FOR FOUNDATIONS

IO. THE CONTRACTOR SHALL OBSERVE WATER CONDITIONS AT THE SITE AND TAKE THE NECESSARY PRECAUTIONS TO ENSURE THAT THE

THE CONTRACTOR SHALL BE RESPONSIBLE FOR COORDINATING THE NEED TO USE FOUNDATION REBAR AS A GROUNDING ELECTRODE

. DESIGN, FABRICATION, AND CONSTRUCTION OF WOOD FRAMING SHALL COMPLY WITH "THE NATIONAL DESIGN SPECIFICATION OF WOOD

MINIMUM STRESS RATING: EXCEPT WHERE INDICATED AS "NON-STRESS RATED", PROVIDE TIMBER WHICH HAS BEEN EITHER GRADED OR

DRESSING: PROVIDE TIMBER WHICH HAS BEEN DRESSED ON 4 SIDES (S4S) AT MILL, PRIOR TO GRADING. COMPLY WITH GRADE SIZES.

8. PSL (PARALLAM) SHALL BE OF WIDTH AND DEPTH AS SPECIFIED ON DRAWINGS, MULTIPLE PLY MEMBERS SHALL BE ASSEMBLED IN

ACCORDANCE WITH THE MANUFACTURERS ASSEMBLY DETAILS. THE FOLLOWING MINIMUM STRUCTURAL PROPERTIES SHALL APPLY:

TESTED AND CERTIFIED WITH ALLOWABLE STRESS RATINGS BASED ON DOUGLAS FIR-LARCH #2 (PSI) OF: Fb = 900, Ft = 575, Fc = 1350,

KEEP STRUCTURAL TIMBER PROTECTED DURING DELIVERY, STORAGE, HANDLING AND ERECTION. DO NOT STORE IN AREAS EITHER

SYSTEM AND SHALL BE RESPONSIBLE FOR INSTALLING THE BONDING CLAMP PRIOR TO PLACEMENT OF THE CONCRETE AS PER NJUCC

4. EXCAVATION SHALL BE PERFORMED SO AS NOT TO DISTURB EXISTING ADJACENT BUILDINGS, STREETS, AND UTILITY LINES. VERIFY

6. REMOVE EXISTING VEGETATION, TOPSOIL, AND UNSATISFACTORY SOILS MATERIALS. PROOF ROLL SUBGRADE TO OBTAIN UNIFORMLY

LOCATION OF ALL UTILITIES PRIOR TO COMMENCEMENT OF WORK. HAND EXCAVATE AROUND UTILITIES AS REQUIRED.

SHALL BE POURED ON THE SAME DAY THE SUBGRADE IS APPROVED BY THE GEOTECHNICAL ENGINEER.

9. UTILITY LINES SHALL NOT BE PLACED THROUGH OR BELOW FOUNDATIONS WITHOUT THE STRUCTURAL ENGINEER'S APPROVAL.

AND RECOMMENDATIONS FOR THE COMMON SOIL TYPE ENCOUNTERED IN THIS AREA.

ADEQUATE BEARING MATERIALS COMPLY WITH BORING LOGS AND DESIGN CRITERIA.

ARCHITECT OUTLINING THE WORK PERFORMED AND TEST RESULTS.

Fc = 850 PSI (PARALLEL TO GRAIN) 12. PLYWOOD FOR ROOF SHEATHING SHALL BE MINIMUM 3/4" FOR FLAT ROOFS AND ROOFS WITH SUPPORTS GREATER THAN 16" O.C. SPACING AND 5/8" MINIMUM FOR SLOPED ROOFS (GREATER THAN 30°) WITH SUPPORTS NO MORE THAN 16" O.C. SPACING AND EACH SHALL CONFORM TO APA PSI RATED SHEATHING, EXTERIOR, 48" X 96" PLYWOOD SHALL BE THREE SPAN CONTINUOUS. FACE GRAIN SHALL BE PERPENDICULAR TO SUPPORTS. PROVIDE ONE PANEL EDGE CLIP BETWEEN SUPPORTS. 13. PLYWOOD FOR FLOOR SHEATHING SHALL BE MINIMUM 3/4" AND EACH SHALL CONFORM TO APA PSI RATED SHEATHING, 48" X 96" PLYWOOD SHALL BE THREE SPAN CONTINUOUS. FACE GRAIN SHALL BE PERPENDICULAR TO SUPPORTS.

14. EXTERIOR WALL PLYWOOD SHEATHING SHALL BE MINIMUM 15/32" AND EACH SHALL CONFORM TO APA PSI RATED SHEATHING 32/16. EXTERIOR. 48" X 46" PLYWOOD SHALL BE THREE SPAN CONTINUOUS. FACE GRAIN SHALL BE PERPENDICULAR TO SUPPORTS. REFER TO TYPICAL DETAILS FOR FASTENING AND LAPPING REQUIREMENTS FLOOR TO FLOOR. MINIMUM EDGE PANEL FASTENING SPACING SHALL BE 6" AND 12" O.C. FIELD SPACING, RE: SHEAR WALL PLAN, IF APPLICABLE, FOR ADDITIONAL REQUIREMENTS. 15. SEE THE INTERNATIONAL BUILDING CODE FOR MINIMUM BRACING AND NAILING REQUIREMENTS 16. PROVIDE AN ADDITIONAL JOIST UNDER PARALLEL NON-LOAD BEARING PARTITIONS THAT SPAN MORE THAN 1/3 THE SPAN OF THE JOIST

17. ALL JOISTS AND RAFTERS SHALL BE RIGIDLY BRACED AT INTERVALS NOT EXCEEDING 6'-O" ON CENTER WITH BRIDGING TO MATCH 18. THE WOOD STRUCTURE IS A NON-SELF SUPPORTING FRAME AND IS DEPENDENT UPON DIAPHRAGM ACTION OF THE PANELS AND ATTACHMENT TO THE SHEAR WALLS FOR STABILITY AND FOR RESISTANCE TO WIND AND SEISMIC FORCES, PROVIDE ALL TEMPORARY SUPPORTS REQUIRED FOR STABILITY AND FOR RESISTANCE TO WIND AND SEISMIC FORCES UNTIL THESE ELEMENTS ARE COMPLETE AND CAST-IN-PLACE CONCRETE CONCRETE SHALL BE DESIGNED AND DETAILED IN ACCORDANCE WITH THE BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE

(ACI 318-19), AND CONSTRUCTED IN ACCORDANCE WITH THE CRSI MANUAL OF STANDARD PRACTICE. CONCRETE IN THE FOLLOWING AREAS SHALL HAVE NATURAL SAND FINE AGGREGATE AND NORMAL WEIGHT COARSE AGGREGATES CONFORMING TO ASTM C33, TYPE I PORTLAND CEMENT CONFORMING TO ASTM C150, AND SHALL HAVE THE FOLLOWING COMPRESSIVE STRENGTH (FC') AT 28 DAYS:

SLABS ON GRADE......4000 PSI AIR ENTRAINMENT 4% TO 6% IN ALL EXPOSED CONCRETE.
MAXIMUM AGGREGATE SIZE SHALL BE I-I/2" FOR FOOTINGS AND 3/4" FOR WALLS AND SLABS

3. THE CONCRETE SUPPLIER SHALL SUBMIT MIX DESIGNS FOR REVIEW, COMPRESSIVE STRENGTH MUST BE SUBSTANTIATED BY A SUITABLE EXPERIENCE RECORD OR BY THE METHOD OF LABORATORY TRIAL BATCHES, THE PERTINENT CRITERIA OF CHAPTER 26 OF ACI 318-14 SHALL APPLY TO THE PROPORTIONING OF MIX DESIGNS AND TO THE ACCEPTANCE OF CONCRETE PRODUCED FOR THE JOB, IF DURING CONSTRUCTION ANY CLASS CONCRETE FAILS TO MEET THE ACCEPTANCE CRITERIA, THE CONTRACTOR SHALL TAKE SUCH STEPS AS ARE DEEMED NECESSARY BY THE STRUCTURAL ENGINEER TO IMPROVE SUBSEQUENT TEST RESULTS AT NO ADDITIONAL COST TO THE OWNER. THE CONTRACTOR SHALL ALSO BEAR THE COST OF SPECIAL INVESTIGATION, TESTING, OR REMEDIAL WORK NECESSARY BECAUSE OF EVIDENCE OF LOW STRENGTH OR NON-CONFORMING CONCRETE OR WORKMANSHIP.

4. MAXIMUM WATER/CEMENT RATIOS:

BEAMS AND COLUMNS

.....4000 P

7. THE OWNER SHALL RETAIN THE SERVICES OF A PROFESSIONAL GEOTECHNICAL ENGINEER, SUBJECT TO THE APPROVAL OF THE ARCHITECT, TO PERFORM SOIL TESTING AND INSPECTION. THE ENGINEER SHALL INSPECT THE SUBGRADE TO VERIFY BEARING LEVELS AND ENSURE B. INTERIOR SLABS C. EXTERIOR SLABS 0.44 CONCRETE SHALL BE NORMAL WEIGHT CONCRETE (144 PCF +) WITH ALL CEMENT CONFORMING TO ASTM CI5O, TYPE I. MAXIMUM PREGATE SIZE SHALL BE 1-1/2" FOR FOOTINGS AND 3/4" FOR WALLS AND SLABS, CONFORMING TO ASTM C330. CONCRETE REINFORCEMENT BARS SHALL CONFORM TO ASTM A615/A-04a, GRADE 60. NO. 3 BARS MAY CONFORM TO ASTM A615/A-04a,

GRADE 40, UNLESS NOTED OTHERWISE. THE "N" DESIGNATION SHALL BE ACCEPTED IN LIEU OF THE "S" DESIGNATION REQUIREMENT, HOWEVER, OTHER REQUIREMENTS SHALL BE MET. REINFORCEMENT BARS SHALL NOT BE TACK WELDED, WELDED, HEATED OR CUT UNLESS INDICATED ON THE CONTRACT DOCUMENTS OR APPROVED BY THE STRUCTURAL ENGINEER 7. WELDED WIRE FABRIC WHEN USED SHALL CONFORM TO ASTM A185. FABRIC SHALL BE SUPPLIED IN FLAT SHEETS, FABRIC SHALL BE 3. GROUT SHALL BE NONSHRINKABLE GROUT CONFORMING TO ASTM C827, AND SHALL HAVE SPECIFIED COMPRESSIVE STRENGTH AT 28

. MINIMUM CONCRETE COVER PROTECTION FOR REINFORCEMENT BARS SHALL BE AS FOLLOWS; (SEE ACI 318-19 SECTION 20.5 FOR CONDITIONS NOT NOTED) A. CONCRETE CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH B. CONCRETE EXPOSED TO EARTH OR WEATHER #6 BARS AND LARGER #5 BARS AND SMALLER C. CONCRETE NOT EXPOSED TO WEATHER NOR IN CONTACT WITH GROUND SLABS, WALLS, JOISTS #II BARS AND SMALLER

DAYS OF 5000 PSI. PREGROUTING OF BASE PLATES WILL NOT BE PERMITTED.

PRIMARY REINFORCEMENT, TIES, STIRRUPS, OR SPIRALS

IO. ALL REINFORCEMENT SHALL BE SECURELY HELD IN PLACE WHILE PLACING CONCRETE. IF REQUIRED, ADDITIONAL BARS, STIRRUPS OR CHAIRS SHALL BE PROVIDED BY THE CONTRACTOR TO FURNISH SUPPORT FOR ALL BARS. II. PLACING OF CONCRETE SHALL NOT START UNTIL THE PLACEMENT OF REINFORCING HAS BEEN APPROVED BY THE INSPECTION AGENCY. 4. ALL GRADES OF TIMBER MUST FULFILL THESE REQUIREMENTS FOR SPECIES, STRESS RATINGS, MOISTURE CONTENT AND OTHER PROVISIONS 12. BONDING AGENT SHALL BE USED WHERE NEW CONCRETE IS PLACED AGAINST EXISTING CONCRETE.

> BE SPACED CLOSER THAN 3 DIAMETERS ON CENTER, ALUMINUM CONDUITS SHALL NOT BE PLACED IN CONCRETE, NO CONDUITS SHALL BE PLACED IN SLABS WITHIN 12 INCHES OF COLUMN FACE OR FACE OF BEARING WALL, NO CONDUITS MAY BE PLACED IN EXTERIOR SLABS . ALL INGERTS AND SLEEVES SHALL BE CAST-IN-PLACE WHENEVER FEASIBLE, DRILLED OR POWDER DRIVEN FASTENERS WILL BE PERMITTED WHEN PROVEN TO THE SATISFACTION OF THE STRUCTURAL ENGINEER THAT THE FASTENERS WILL NOT SPALL THE CONCRETE

13. PIPES OR CONDUITS PLACED IN SLABS SHALL NOT HAVE AN OUTSIDE DIAMETER LARGER THAN 1/3 THE SLAB THICKNESS AND SHALL NOT

. MOISTURE CONTENT: EXCEPT AS OTHERWISE INDICATED, PROVIDE TIMBER DRIED TO MAXIMUM MOISTURE CONTENT OF 19%, AND INCLUDE AND HAVE THE SAME CAPACITY AS CAST-IN-PLACE INSERTS, WHEN INSTALLING EXPANSION BOLTS OR ADHESIVE ANCHORS, THE CONTRACTOR SHALL TAKE MEASURES TO AVOID DRILLING OR CUTTING OF ANY EXISTING REINFORCING AND DESTRUCTION OF CONCRETE. HOLES SHALL BE BLOWN CLEAN PRIOR TO PLACING BOLTS OR ADHESIVE ANCHORS. 15. MIXING, TRANSPORTING AND PLACING OF CONCRETE SHALL CONFORM TO ACI REQUIREMENTS.

16. WELDING OF REINFORCEMENT BARS, WHEN APPROVED BY THE STRUCTURAL ENGINEER, SHALL CONFORM TO THE AMERICAN WELDING SOCIETY STANDARD DI.1-15 AND DI.4-18. ELECTRODES FOR SHOP AND FIELD WELDING OF REINFORCEMENT BARS SHALL CONFORM TO

17. HORIZONTAL JOINTING WILL NOT BE PERMITTED IN CONCRETE CONSTRUCTION EXCEPT AS SHOWN ON THE CONTRACT DOCUMENT. VERTICAL JOINTS SHALL OCCUR AT CENTER OF SPANS AT LOCATIONS APPROVED BY THE STRUCTURAL ENGINEER. 18. SLABS SHALL HAVE CONSTRUCTION JOINTS OR CRACK CONTROL JOINTS AT EACH COLUMN LINE IN EACH DIRECTION, (REFER TO PLAN)
ADDITIONAL CRACK CONTROL JOINTS SHALL BE PROVIDED, SUCH THAT THE MAXIMUM SPACING BETWEEN CONSTRUCTION AND/OR CRACK

CONTROL JOINTS DOES NOT EXCEED 30X SLAB THICKNESS IN INCHES AND LENGTH TO WIDTH RATIO 1.5:1.

19 REPAIR CONCRETE EXHIBITING VOIDS DUE TO SNAP TIES "HONEYCOMBS" ROCK POCKETS AND RUNS SPALLS OR OTHERWISE DAMAGED SURFACES WITH DRY PACK OR CEMENT GROUT, AND FINISH FLUSH WITH ADJOINING SURFACES. AT THE DISCRETION OF THE STRUCTURAL ENGINEER OR AS QUALIFIED BY LAB TESTING, EXCESSIVE HONEYCOMBS OR EXPOSED REINFORCEMENT THAT JEOPARDIZE THE DESIGN SHALL BE REMOVED AND REPLACED AT THE EXPENSE OF THE CONTRACTOR. 20. CONSTRUCTION JOINTS BETWEEN FOOTINGS AND PILASTERS AND SIMILAR JOINTS SHALL BE PREPARED BY ROUGHENING THE CONTACT

SURFACE IN AN APPROVED MANNER TO A FULL AMPLITUDE OF APPROX. I/4 INCHES, LEAVING THE CONTACT SURFACE FREE AND CLEAR OF LAITANCE. REINFORCED (DOWELLED) JOINTS SHALL HAVE BINDER ADDITIVE APPLIED PRIOR TO POUR. 21. ALL EXPOSED CORNERS SHALL BE CHAMFERED 3/4" UNLESS OTHERWISE INDICATED.

22. CONTRACTOR SHALL TAKE EVERY PRECAUTION TO PROTECT FINISHED SURFACES FROM STAINS OR ABRASIONS. NO FIRE SHALL BE ALLOWED IN DIRECT CONTACT WITH CONCRETE. PROVIDE ADEQUATE PROTECTION AGAINST INJURIOUS ACTION BY SUN OR WIND, FRESH CONCRETE SHALL BE THOROUGHLY PROTECTED FROM HEAVY RAIN, FLOWING WATER, AND MECHANICAL INJURY. 23. TOPS OF FOUNDATIONS SHALL BE TROWEL FINISHED AND SMOOTH, REFER TO DRAWINGS FOR BASE PLATE ACCOMMODATIONS.

24, WATER SHALL NOT BE ADDED TO THE CONCRETE AT THE JOBSITE, IT SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR TO

ADDITION OF WATER AT THE JOBSITE, THE USE OF PLASTICIZERS, RETARDANTS AND OTHER ADDITIVES SHALL BE AT THE JOBSITE. THE USE OF PLASTICIZERS, RETARDANTS AND OTHER ADDITIVES SHALL BE AT THE OPTION OF THE ONTRACTOR SUBJECT TO THE APPROVAL OF THE STRUCTURAL ENGINEER. FOLLOM THE RECOMMENDATIONS OF THE MANUFACTURER FOR PROPER USE OF RETARDANTS AND OTHER ADDITIVES, USE OF CALCIUM E CHLORIDE OR OTHER CHLORIDE BEARING SALTS SHALL NOT BE

CONCRETE HAS LOST SURFACE WATER SHEEN OR ALL FREE SLAB SURFACE, FINISHING OF SLAB SURFACES SHALL COMPLY WITH ACI 26. PROVIDE 7 DAY CURING IMMEDIATELY AFTER FINISHING USING ONE OF THE FOLLOWING METHODS: A. CONTINUOUSLY WATERED BURLAP

B. WATERPROOF MEMBRANES C. SPRAYED-ON LIQUID MEMBRANE 27. REFER TO THE MANUFACTURER'S SPECIFICATIONS FOR REQUIREMENTS. PROTECT THE CONCRETE SURFACE BETWEEN FINISHING OPERATIONS

ON HOT, DRY DAYS OR ANY TIME PLASTIC SHRINKAGE CRACKS DEVELOP USING WET BURLAP, PLASTIC MEMBRANES OR FOGGING. PROTECT CONCRETE DECK AT ALL TIMES FROM RAIN, HAIL OR OTHER INJURIOUS EFFECTS 28. SLABS SHALL BE REINFORCED WITH WELDED WIRE FABRIC AT -I" FROM TOP OF SLAB. THE WELDED WIRE FABRIC SHALL BE SECURELY HELD IN PLACE WITH ADDITIONAL BARS, STIRRUPS OR CHAIRS. 29. PROVIDE POUR STOP MATERIAL WHERE NOT INDICATED ON PLAN AS REQUIRED TO COMPLETE JOB.

30. HOT WEATHER CONCRETING: WHEN CONCRETING IS TO BE DONE IN HOT WEATHER CONDITIONS THAT COULD ADVERSELY AFFECT THE PROPERTIES AND SERVICEABILITY OF CONCRETE, PREPARATIONS AND PROCEDURES OUTLINED IN ACI 305R-05 SHOULD BE FOLLOWED UNLESS OTHERWISE NOTED IN CONSTRUCTION SPECIFICATIONS. 31. COLD WEATHER CONCRETING: WHEN CONCRETING IS TO BE DONE IN COLD WEATHER CONDITIONS THAT COULD ADVERSELY AFFECT THE PROPERTIES AND SERVICEABILITY OF CONCRETE, PREPARATIONS AND PROCEDURES OUTLINED IN ACI 306R-05 SHOULD BE FOLLOWED

UNLESS OTHERWISE NOTED IN CONSTRUCTION SPECIFICATIONS.

STRUCTURAL STEEL FABRICATION AND ERECTION OF STRUCTURAL STEEL SHALL CONFORM TO "THE STEEL CONSTRUCTION MANUAL", FIFTEENTH EDITION, AMERICAN INSTITUTE OF STEEL CONSTRUCTION INCLUDING THE 2016 SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS, THE 2014 RCSC

SPECIFICATION FOR STRUCTURAL JOINTS USING HIGH STRENGTH BOLTS AND ANSI/AISC 303-16 CODE OF STANDARD PRACTICE. 2. WIDE FLANGE SHAPES: ASTM A992 OR A572, GRADE 50. 3. STRUCTURAL SHAPES & PLATES: ASTM A36, A572 OR A992. 4. STEEL TUBING (SQUARE, RECT., OR ROUND): ASTM A500, GRADE B. 5. GALVANIZED STRUCTURAL STEEL

A. STRUCTURAL SHAPES AND RODS:

B. BOLTS, FASTENERS AND HARDWARE:

ANCHOR RODS SHALL CONFORM TO ASTM F1554, GRADE 55, OR ASTM A307 UNLESS NOTED OTHERWISE CONNECTION BOLTS FOR STRUCTURAL STEEL MEMBERS SHALL MEET OR EXCEED THE REQUIREMENTS OF ASTM A325. BOLTS SHALL BE DESIGNED AS BEARING TYPE BOLTS, EXCEPT AS NOTED HEREIN OR ON PLAN. BEARING BOLTS SHALL BE INSTALLED IN ACCORDANCE WITH THE "SNUG TIGHT" CONDITION AS OUTLINED IN THE AISC SPECIFICATIONS FOR STRUCTURAL JOINTS USING ASTM A325 OR A490 BOLTS. BOLTS IN BRACING CONNECTIONS MOMENT CONNECTIONS OR OTHER CONNECTIONS NOTED ON THE DRAWINGS ARE CONSIDERED TO BE

SLIP-CRITICAL BOLTS AND SHALL BE TIGHTENED BY THE TURN-OF-NUT METHOD OR SHALL UTILIZE LOAD INDICATOR TYPE BOLTS, INSTALLED IN ACCORDANCE WITH THE RECOMMENDATIONS OF THE MANUFACTURER. CONNECTION BOLTS SHALL HAVE HARDENED WASHER PLACED UNDER THE ELEMENT TO BE TIGHTENED. 6. THE FABRICATOR IS RESPONSIBLE FOR THE SELECTION, DESIGN AND DETAILING OF ALL CONNECTIONS NOT FULLY DETAILED IN THE CONTRACT DOCUMENTS. TYPICAL CONNECTION DETAILS ARE INDICATED ON THE DRAWINGS FOR DESIGN INTENT ONLY. THE FABRICATOR SHALL HAVE A REGISTERED PROFESSIONAL ENGINEER PREPARE THE CONNECTION DESIGNS, AND SUCH DESIGNS SHALL BE SUBMITTED FOR REVIEW WITH THE SHOP DRAWINGS, CONNECTIONS SHALL BE DESIGNED AND DETAILED IN ACCORDANCE WITH THE AISC "MANUAL OF STEEL CONSTRUCTION" FIFTEENTH EDITION. PROVIDE DOUBLE ANGLE CONNECTIONS OR KNIFE PLATE CONNECTIONS FULL DEPTH OF THE SUPPORTING BEAM UNLESS NOTED OTHERWISE, MINIMUM (2) BOLTS PER BEAM, ALL CONNECTIONS SHALL BE DESIGNED AND DETAILED TO DEVELOP A MINIMUM END REACTION OF 12.0 KIPS UNLESS NOTED OTHERWISE ON PLAN. ALL BEAM TO COLUMN CONNECTIONS SHALL BE

AND COMPRESSION). ALL FORCES LISTED ARE ALLOWABLE STRESS LEVEL FORCES UNLESS NOTED OTHERWISE. 1. ALL CONNECTIONS SHALL BE SYMMETRICAL ABOUT THE AXIS OF THE MEMBER CONNECTED. PROVIDE ONLY ONE GRADE OF BOLT FOR EACH BOLT DIAMETER TO BE USED IN THE CONNECTIONS. DO NOT MIX GRADE OF BOLTS.

IO. PRIOR TO DETAILING CONNECTIONS FOR STRUCTURAL STEEL, THE STEEL FABRICATOR SHALL SUBMIT FOR REVIEW REPRESENTATIVE DETAILS FOR EACH TYPE OF STRUCTURAL STEEL CONNECTION TO BE UTILIZED. AFTER REVIEW, THE CONNECTIONS MAY BE INCORPORATED INTO SHOP DRAWINGS, ALONG WITH A TABLE OF DESIGN CAPACITIES FOR THE RANGE OF CONNECTIONS TO BE USED.

DESIGNED FOR THE MINIMUM SHEAR REACTION INDICATED ABOVE IN COMBINATION WITH A 10 KIP AXIAL FORCE (ACTING IN BOTH TENSION

. WELDING SHALL CONFORM TO THE AMERICAN WELDING SOCIETY STANDARD DI.I-15, I-80 ELECTRODES FOR SHOP AND FIELD WELDS SHALL CONFORM TO AMS A5.1 OR AMS A5.5 CLASS ETOXX, LOW HYDROGEN. MINIMUM WELD SIZE SHALL BE 3/16" UNLESS NOTED OTHERWISE.

12. CUTS, HOLES, COPING, ETC. REQUIRED FOR OTHER TRADES OR FIELD CONDITIONS SHALL BE SHOWN ON THE SHOP DRAWINGS AND MADE IN THE SHOP. CUTTING OR BURNING OF MAIN STRUCTURAL MEMBERS IN THE FIELD WILL NOT BE PERMITTED. 13. STEEL MEMBERS SHOWN ON PLAN SHALL BE EQUALLY SPACED UNLESS NOTED OTHERWISE

14. THE GENERAL CONTRACTOR AND STEEL ERECTOR SHALL NOTIFY THE STRUCTURAL ENGINEER OF ANY FABRICATION OR ERECTION ERRORS OR DEVIATIONS AND RECEIVE WRITTEN APPROVAL BEFORE ANY FIELD CORRECTIONS ARE MADE. 15. WELDING TO THE EXISTING STEEL WILL NOT BE ALLOWED AND THE CONTRACTOR SHALL ANTICIPATE USING FIELD BOLTED CONNECTIONS TO

16. ALL STEEL SHALL BE PAINTED WITH SHOP STANDARD PRIMER UNLESS NOTED OTHERWISE. 17. STEEL ANGLES AND PLATES ALONG WITH BOLTS AND WASHERS, IN DIRECT CONTACT WITH EXTERIOR FINISH MASONRY, AND ALL EXTERIOR

EXPOSED STRUCTURAL STEEL, SHALL BE PAINTED WITH INORGANIC ZINC PRIMER EQUIVALENT TO SOUTHERN COATINGS CHEMTEC 600 IB. SPLICING OF STRUCTURAL STEEL MEMBERS WHERE NOT DETAILED ON THE CONTRACT DOCUMENTS IS PROHIBITED WITHOUT PRIOR WRITTEN APPROVAL OF THE STRUCTURAL ENGINEER AS TO LOCATION TYPE OF SPLICE AND CONNECTION TO BE MADE

I. PROVIDE LOOSE OR HANGING LINTELS NOT SHOWN ON DRAWINGS AS REQUIRED TO COMPLETE JOB. COORDINATE WITH ARCHITECTURAL AND STRUCTURAL DRAWINGS, CONTACT STRUCTURAL ENGINEER AS PER ANY DESIGN INFORMATION REQUIRED. FIELD WELD ALL ADJUSTABLE LINTEL CONNECTIONS AFTER FINAL ALIGNMENT

20. FIELD WELDED SURFACES WITHIN FOUR (4) INCHES OF WELD SHALL BE CLEANED AND GROUND SMOOTH. AFTER WELDING COAT THE EXPOSED AREA WITH APPROPRIATE PRIMER/PAINTS AS SPECIFIED.

21. IF STEEL IS GALVANIZED, COAT THE EXPOSED AREA WITH GALVANIZING REPAIR PAINT. GALVANIZING REPAIR PAINT SHALL BE A HIGH ZINC DUST CONTENT PAINT COMPLYING WITH FEDERAL SPECIFICATIONS DOD-P-21035A OR SSPC-PAINT-20, COLD GALVANIZING COMPOUND BY ZRC PRODUCTS CO. OR EQUAL.

22. THE STEEL STRUCTURE IS A NON-SELF-SUPPORTING STEEL FRAME AND IS DEPENDENT UPON DIAPHRAGM ACTION OF THE METAL

(ROOF/FLOOR) DECK AND ATTACHMENT TO THE MASONRY WALLS FOR STABILITY AND FOR RESISTANCE TO WIND AND SEISMIC FORCES. PROVIDE ALL TEMPORARY SUPPORTS REQUIRED FOR STABILITY AND FOR RESISTANCE TO WIND AND SEISMIC FORCES UNTIL THESE ELEMENTS ARE COMPLETE AND ARE CAPABLE OF PROVIDING THIS SUPPORT. 23. VISUALLY INSPECT ALL FILLET WELDS. IO% OF ALL FIELD FILLET WELDS IN PRIMARY CONNECTIONS AND MULTI-PASS WELDS SHALL BE TESTED BY THE MAGNETIC PARTICLE METHOD, COMPLYING WITH EIO9, PERFORMED ON THE ROOT PASS AND ON THE FINISHED WELD.

24.100% OF FULL PENETRATION WELDS SHALL HAVE ULTRASONIC INSPECTION, COMPLYING WITH ASTM E164 25. DELETE PAINT ON ALL STEEL TO RECEIVE SPRAYED-ON FIREPROOFING OR CONCRETE ENCASEMENT.

26. ALL DISSIMILAR METALS SHALL BE TREATED OR PROPERLY SEPARATED TO PREVENT GALVANIC AND/OR CORROSIVE EFFECTS.

Norwescap Old Sullivan Building Remodeling Project

PROJECT LOCATION

LOT:

BLOCK:

371 S Main Street Phillipsburg, NJ 08865 Warren County

350 Marshall Street

Phillipsburg, NJ 08865



312 State Route 10, Randolph, NJ 07869 Tel: 973.442.5880 Fax: 973.442.5886



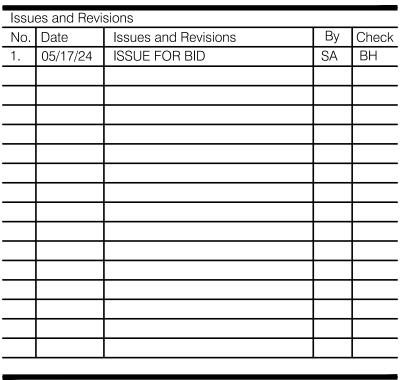
LICENSE NUMBER: GE 51243 OF THE ENGINEER THE REPRODUCTION OF THIS DRAWI SAID DRAWING SHALL BE CONSIDERED A VIOLATION OF BOTH THE PROFESSIONAL CODE OF ETHICS AND A THEFT OF COMPANY ASSETS, BOTH OF WHICH SHALL BE DESSECTIFED TO THE FILL IS TEVETENT OF

PERSECUTED TO THE FULLEST EXTENT OF CURRENT STATUTES.

P.O. BOX 628 FARMINGDALE, NJ 07727 732.938.2666 732.938.2661

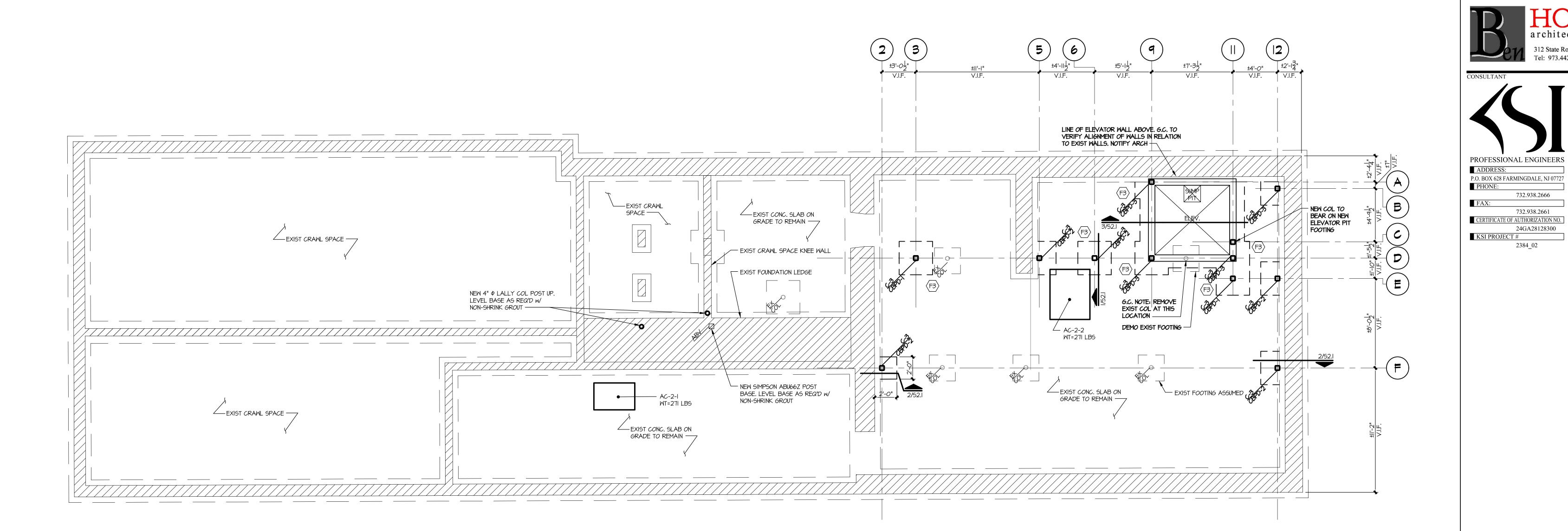
CERTIFICATE OF AUTHORIZATION NO.

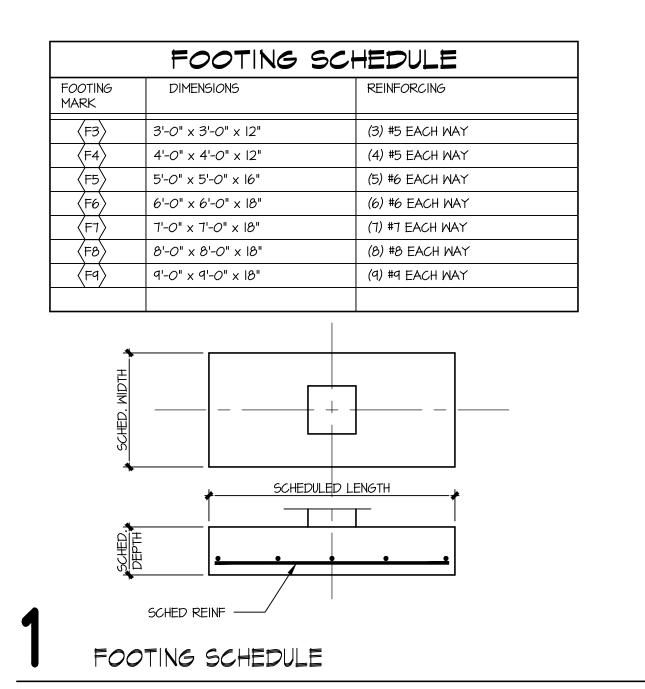
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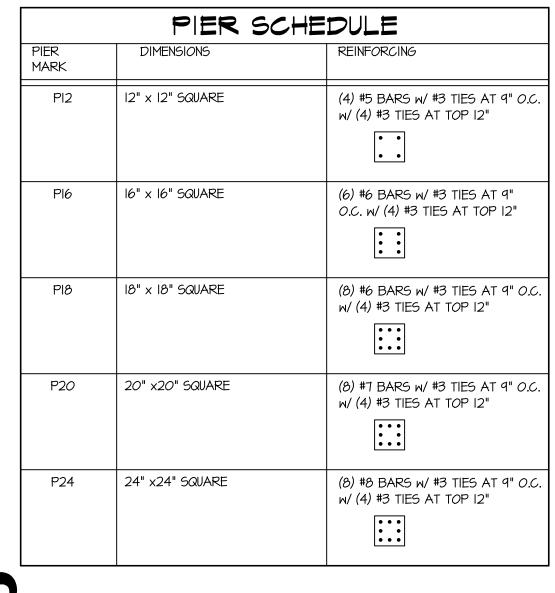


NJ License No: AI 15794 Benjamin J. Horten

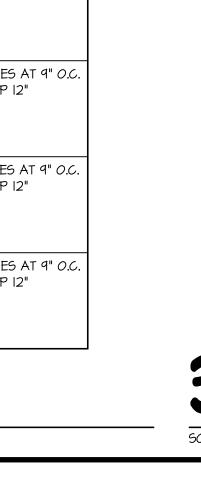
\\ksi-far-dc-002\\data\2023 Projects\2384 Ben Horten\2384 02 371 Main St., Phillipsburg_Structural Drawings\2384_02_Details_B.dwg

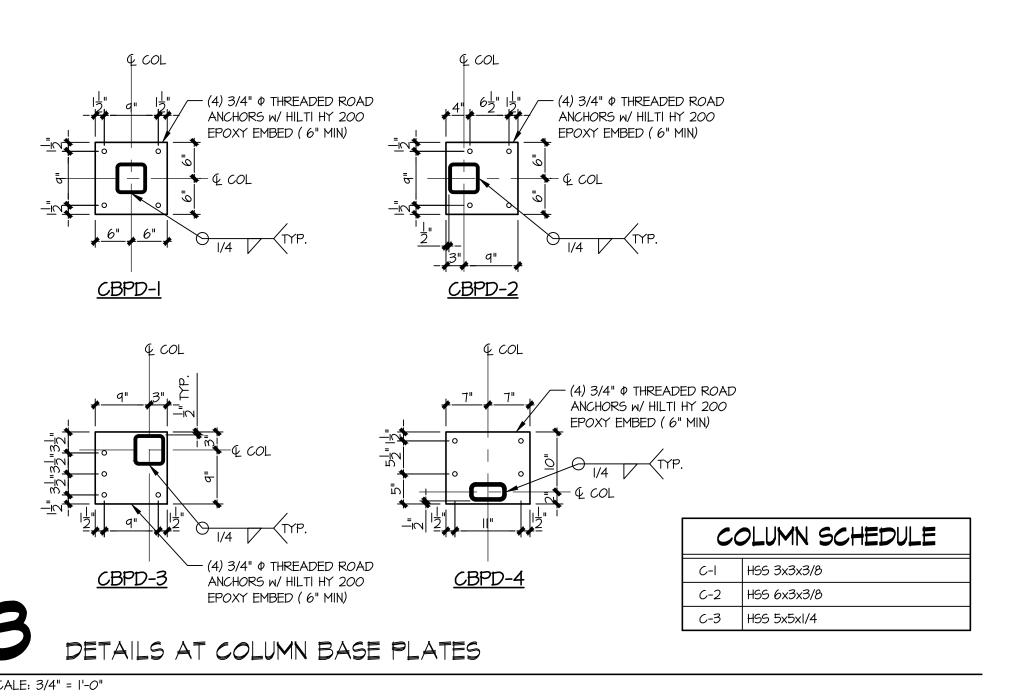






PIER SCHEDULE







- I. THE CONTRACTOR SHALL VERIFY ALL EXISTING BUILDING INFORMATION SHOWN (DIMENSIONS, ELEVATIONS, ETC) AND NOTIFY THE ARCHITECT/ENGINEER OF ANY DISCREPANCIES PRIOR TO FABRICATION OF ANY STRUCTURAL COMPONENT. FAILURE TO
- NOTIFY ARCHITECT/ENGINEER OF UNSATISFACTORY CONDITIONS CONSTITUTES ACCEPTANCE OF UNSATISFACTORY CONDITIONS. 2. G.C. IS RESPONSIBLE TO FIELD VERIFY EXIST DIMENSIONS AND MEMBER SIZE. CONTACT ENGINEER IMMEDIATELY WITH ANY SIGNIFICANT DIFFERENCES TYPICAL AT ALL FLOORS. 3. PROVIDE TEMPORARY SHORING WHERE REQ'D TO MAINTAIN STABILITY.
- 4. NEW SLAB ON GRADE SHALL BE 4" NORMAL WEIGHT CONCRETE (f'c = 4,000 psi AT 28 DAYS) PLACED OVER A VAPOR BARRIER ON 6" OF CRUSHED STONE. REINFORCE WITH 6x6-WI.4xWI.4 WWF. I" FROM TOP OF SLAB, SUPPORTED ON CHAIRS.
- 5. FOOTINGS SHALL BEAR ON VIRGIN SOIL OR CONTROLLED COMPACTED FILL HAVING A MINIMUM BEARING CAPACITY OF 2,000 POUNDS PER SQUARE FOOT. TOP OF FOOTING ELEVATIONS ARE SHOWN THUS -x" ON PLAN AND ARE REFERENCED TO THE BUILDING'S
- 6. TOP OF PIERS ARE SHOWN THUS +X'-X" T.O.P. ON PLAN AND ARE REFERENCED TO THE BUILDING'S DATUM.
- 7. FX INDICATES FOOTING TYPE, SEE FOOTING SCHEDULE FOR SIZE AND REINFORCING. 8. ALL COLUMN FOOTINGS SHALL BE CENTERED UNDER COLUMN CENTERLINES UNLESS NOTED OTHERWISE ON PLAN.
- 9. FOR TYPICAL DETAILS SEE DRAWING S4.I 10. FOR GENERAL NOTES SEE DRAWING SO.I
- II. (F.S). DENOTES FOOTING STEP. SEE DETAIL SHEETS. 12. C.J. DENOTES CONTROL JOINT. RE: TYPICAL DETAILS FOR CONSTRUCTION. 13. AT ALL LOCATIONS WHERE PLUMBING LINES EXIT THE BUILDING, DEEPEN FOOTINGS AS REQUIRED TO ALLOW 6" (MIN) CLEARANCE BETWEEN PIPE AND FOOTING.

No. Date Issues and Revisions . 05/17/24 | ISSUE FOR BID

Norwescap

Old Sullivan Building

Remodeling Project

371 S Main Street

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Warren County

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732.938.2666

732.938.2661

BLOCK:

PROJECT LOCATION

LOT:

Benjamin J. Horten	NJ License No: AI 15794		
Drawing Description:			
FOUNDATION PLAN			

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	HEADER SCHEDULE
	H-I (2) 2x8'5
	H-2 (2) 2xIO'5
	H-3 (2) 2xI2'5
1	H-4 (3) 2x8'5
	H-5 (3) 2xIO'5
	H-6 (3) 2xl2'5
Ľ	

	CALENIE
COLUMN	SCHEDULE

C-I	HSS 3x3x3/8
C-2	HSS 6x3x3/8
C-3	HSS 5x5x1/4

F	POST SCHEDULE
P-I	(2) 2x4'5
P-2	(2) 2x6'5
P-3	(3) 2x4'S
P-4	(3) 2x6'5
P-5	4x4
P-6	6x6
P-7	3-1/2x3-1/2 PSL POST
P-8	3-1/2x5-1/4 PSL POST
P-9	5-1/4x5-1/4 PSL POST

BEAM SCHEDULE

	B-I	I-3/4x9-I/2 LVL
	B-2	3-1/2x9-1/2 PSL
	B-3	5-1/4x9-1/2 PSL
	B-4	7x9-1/2 PSL
	B-5	I-3/4xII-7/8 LVL
	B-6	3-1/2x11-7/8 PSL
	B-7	5-1/4x11-7/8 PSL
	B-8	7xII-7/8 PSL
	B-9	I-3/4xI4 LVL
	B-10	3-1/2x14 PSL
	B-II	5-1/4x14 PSL
	B-I2	7xI4 PSL
	B-I3	I-3/4xI6 LVL

B-14 3-1/2x16 PSL B-15 5-1/4x16 PSL B-16 7x16 PSL B-18 5-1/4x18 PSL

B-19 5-1/4x20 PSL

SEE STRUCTURAL NOTES FOR REQUIRED WOOD SPECIES AND 2. PROVIDE I/2" PLYWOOD SHIM BETWEEN EACH PLY. MATCH DEPTH

OF HEADER 3. FOR 2x8 MULTIPLE MEMBERS GLUE AND NAIL EACH PLY W/ (3) ROWS OF 16d NAILS AT 8" O.C.

4. FOR 2xIO AND 2xI2 MULTIPLE MEMBERS GLUE AND NAIL EACH PLY W/ (4) ROWS OF 16d NAILS AT 8" O.C.

. NAIL OR BOLT MULTIPLE LVL BEAMS AND HEADERS PER MANUFACTURERS REQUIREMENTS.

6. PRE-ENGINEERED WOOD HEADERS MAY BE SUBSTITUTED FOR THE 2x WOOD HEADERS SHOWN ON THE DRAWINGS. THE CONTRACTOR SHALL SUBMIT SHOP DRAWINGS FOR APPROVAL PRIOR TO CONSTRUCTION.

HANGER SCHEDULE DIMENSIONAL LUMBER (2) 2x8'5 LUS26-2 (2) 2x10'5 LUS28-2 (2) 2xl2'5

HANGER SCHEDULE WOOD I-JOISTS

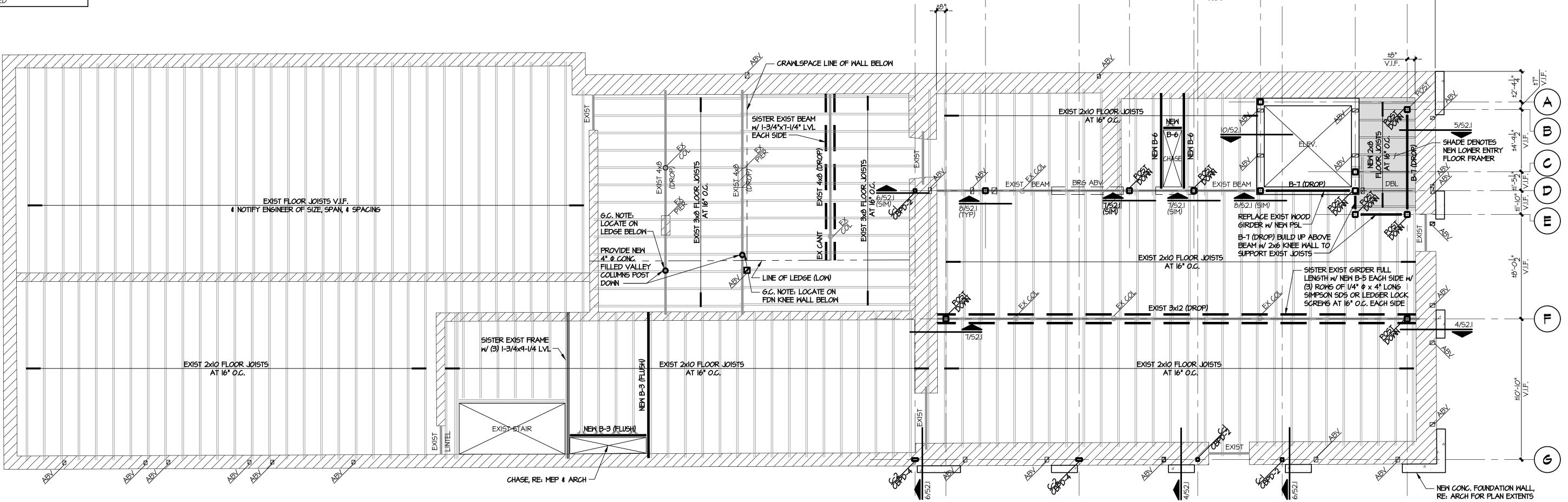
71005				
JOIST	TTL FLANGE WIDTH	HANGER		
(1) 9-1/2 110	l 3/4"	ITSI.8I/9.5		
(1) 9-1/2 230	2 5/16"	ITS2.37/9.		
(1) 11-7/8 110	l 3/4"	ITSI.8I/II.8		
(1) 11-7/8 230/360	2 5/16"	ITS2.37/II.		
(1) 11-7/8 560	3 1/2"	ITS3.56/II		
(1) 14 230/360	2 5/16"	ITS2.37/I4		
(I) 14 56 <i>0</i>	3 1/2"	ITS3.56/I4		
(2) 9-1/2 110	3 1/2"	MIT49.5		
(2) 9-1/2 230	4 5/8"	MIT359.5-		
(2) 11-7/8 110	3 1/2"	MIT4II.88		
(2) 11-7/8 230/360	4 5/8"	MIT3511.88		
(2) 11-7/8 560	7"	HB7.12/11.8		
(2) 14 230/360	4 5/8"	MIT3514-2		
(2) 14 560	7"	HB7.12/14*		

* REQUIRES WEB STIFFENERS FLITCH BEAM

FB-I | I"x9" STEEL PL w/ (2) I-3/4x9-1/4 LVL w/ (2) I/2" \$\phi\$ THRU BOLTS AT ENDS \$ BALANCE (I) AT I2" O.C.

PRE-ENGINEERED BEAMS 1-3/4x9-1/2 LVL 3-1/2x9-1/2 PSL HB3.56/9.5 5-1/4x9-1/2 PSL HGLTV5.37 H=9.5 1-3/4x11-7/8 LVL MITII.88 HGLTV3.511 3-1/2x11-7/8 PSL HGLTV5.37 H=11.875 5-1/4x11-7/8 PSL 1xII-7/8 PSL HGLTV7 H=11.875 -3/4xI4 LVL -1/2x14 PSL HGLTV3.514 5-1/4x14 PSL HGLTV5.37 H=14 7x14 PSL HGLTV7 H=14 1-3/4x16 LVL MITI.81/16 3-1/2x16 PSL HGLTV3.516 -1/4x16 PSL HGLTV5.37 H=16 7x16 PSL HGLTV7 H=16

HANGER SCHEDULE



G.C. NOTE: IN BID PROVIDE CONC SIDEWALK REPAIRS DUE TO DISTURBANCE CAUSED BY NEW FOUNDATION WALL BUMP OUTS. ALSO, PROVIDE ALLOWANCE FOR DAMPROOFING OF EXIST EXT. BASEMENT WALL 3 FEET DOWN FROM GRADE, COORD SPEC W/ ARCH



FIRST FLOOR FRAMING PLAN

I. THE CONTRACTOR SHALL VERIFY ALL EXISTING BUILDING INFORMATION SHOWN (DIMENSIONS, ELEVATIONS, ETC) AND NOTIFY THE ARCHITECT/ENGINEER OF ANY DISCREPANCIES PRIOR TO FABRICATION OF ANY STRUCTURAL COMPONENT. FAILURE TO NOTIFY ARCHITECT/ENGINEER OF UNSATISFACTORY CONDITIONS CONSTITUTES

- ACCEPTANCE OF UNSATISFACTORY CONDITIONS. 2. G.C. IS RESPONSIBLE TO FIELD VERIFY EXIST DIMENSIONS AND MEMBER SIZE. CONTACT ENGINEER IMMEDIATELY WITH ANY SIGNIFICANT DIFFERENCES TYPICAL AT ALL FLOORS.
- 3. PROVIDE TEMPORARY SHORING WHERE REQ'D TO MAINTAIN STABILITY. 4. TYPICAL FLOOR CONSTRUCTION 3/4" PLYWOOD SHEATHING ON WOOD FRAMED
- STRUCTURE. 5. ALL HEADERS TO BE DROPPED UNLESS NOTED OTHERWISE.
- 6. ALL 2x STRUCTURAL FRAMING TO BE #2 DOUG FIR OR BETTER. 7. ALL JOISTS SHALL HAVE ONE ROW OF BRIDGING AT THE MIDSPAN.
- 8. ALL HEADERS TO BE MIN. (2) 2xIO'S IN 2x4 EXTERIOR WALL AND (3) 2xIO'S IN 2X6 EXTERIOR WALL UNLESS NOTED OTHERWISE ON PLAN.
- 9. REFER TO ARCH SECTIONS AND ELEVATIONS FOR PLATE HEIGHTS.
- IO. FOR TYPICAL DETAILS SEE DRAWING S4.I II. FOR GENERAL NOTES SEE DRAWING SO.I
- 12. GENERAL CONTRACTOR IS RESPONSIBLE TO INSTALL ALL PROPRIETARY FLOOR FRAMING IN ACCORDANCE WITH ALL MANUFACTURER'S REQUIREMENTS. TYPICAL FOR ALL FLOOR AND ROOF FRAMING THAT IS PRE-ENGINEERED LUMBER.
- 13. INSTALL PROPER JOIST HANGERS AT ALL JOIST MEMBERS. THE INSTALLATION OF THE JOIST HANGERS SHALL COMPLY WITH THE MANUFACTURER'S RECOMMENDED DETAILS. 14. PLACE DOUBLE JOISTS UNDER WALLS ABOVE OR PROVIDE SOLID BLOCKING AT 24" O.C. UNDER WALLS ABOVE WHERE NOT ALREADY INSTALLED.
- 15. H-X/P-X DENOTES HEADER/BEAM AND END POST DESIGNATION, RE: SCHEDULE FOR SIZE. POSTS SIZES INDICATED ON PLAN AT POST LOCATIONS ARE SPECIFIC TO HEADER/BEAM END ONLY.

Norwescap Old Sullivan Building Remodeling Project

PROJECT LOCATION

LOT:

BLOCK:

371 S Main Street Phillipsburg, NJ 08865 Warren County

NORWESCAP

350 Marshall Street Phillipsburg, NJ 08865



P.O. BOX 628 FARMINGDALE, NJ 07727

732.938.2666

732.938.2661 CERTIFICATE OF AUTHORIZATION NO.

24GA28128300 KSI PROJECT #

No. Date Issues and Revisions . 05/17/24 | ISSUE FOR BID

NJ License No: AI 15794 Benjamin J. Horten FIRST FLOOR FRAMING PLAN

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Drawings\2384_02_Plans_C.dwg

•						
	HEADER SCHEDULE		ILE HA	HANGER SCHED		
	H-I	(2) 2x8'5	DIM	ENSIONAL LUM		
	H-2	(2) 2x10'5	2x8	LUS26		
	H-3	(2) 2x12'5	(2) 2x8'5	LUS26-2		
	H-4	(3) 2x8'S	2xIO	LUS28		
	H-5	(3) 2x10'5				
	H-6	(3) 2x12'5	(2) 2x10'5	LU528-2		
			2xl2	LUS210		
	_		(2) 2x12'5	LUS210-2		
		OLUMN SCHEDL		•		
	C-I	HSS 3x3x3/8	——————————————————————————————————————	NGER SCHEDU		

(2) 2xl2'5		
	OLUMN SCHEDULE	
HANGER SCH	H55 3x3x3/8	C-I
	HSS 6x3x3/8	C-2
DL-I DOOM	HSS 5x5xI/4	C-3
JOIST TTL FLAN		

C-3	HSS 5x5x1/4	MOOI	7 1-1015	
		JOIST	TTL FLANGE WIDTH	HANGE
F	POST SCHEDULE	(1) 9-1/2 110	1 3/4"	ITSI.8I
P-I	(2) 2x4'5	(1) 9-1/2 230	2 5/16"	IT52.3
P-2	(2) 2x6'5	(1) 11-7/8 110	3/4"	ITSI.8I
P-3	(3) 2x4'5	(1) 11-7/8 230/360	2 5/16"	ITS2.3
P-4	(3) 2x6'5	(1) 11-7/8 560	3 1/2"	ITS3.5
P-5	4x4	(I) 14 230/360	2 5/16"	ITS2.3
P-6	6x6	(1) 14 560	3 1/2"	ITS3.5
P-7	3-1/2x3-1/2 PSL POST	(2) 9-1/2 110	3 1/2"	MIT49
P-8	3-1/2x5-1/4 PSL POST	(2) 9-1/2 230	4 5/8"	MIT35
P-9	5-1/4x5-1/4 PSL POST	(2) 11-7/8 110	3 1/2"	MIT4II
' '	3 1/ 1/3 1/ 1 3 12 1 0 3 1	(2) 11-7/8 230/360	4 5/8"	MIT35
		(2) 11-7/8 560	7"	HB7.12
5	BEAM SCHEDULE	(2) 14 230/360	4 5/8"	MIT35

	5-I/4x9-I/2 PSL		FLIT	CH BEA	M
	3-I/2x9-I/2 PSL	_	* REQUIRES WEB ST	IFFENERS	
	I-3/4x9-I/2 LVL		(2) 14 560	7"	HB7.12/14*
	BEAM SCHEDULE		(2) 14 230/360	4 5/8"	MIT3514-2
			(2) 11-7/8 560	7"	HB7.12/11.8
	5 , M5 , 11 5E 1 651		(2) 11-7/8 230/360	4 5/8"	MIT3511.88
	5-1/4x5-1/4 PSL POST		(2) 11-7/8 110	3 1/2"	MIT4II.88
	3-1/2x5-1/4 P9L P09T		(2) 9-1/2 230	4 5/8"	MIT359.5-
	3-1/2x3-1/2 PSL POST		(2) 9-1/2 110	3 1/2"	MIT49.5
	6×6		(I) 14 56 <i>0</i>	3 1/2"	ITS3.56/I
	4x4		(I) 14 23 <i>0/</i> 36 <i>0</i>	2 5/16"	ITS2.37/I4
	(3) 2×6'5		(1) 11-7/8 560	3 1/2"	ITS3.56/II
	(3) 2x4'5	L	(1) 11-1/0 250/560	2 5/16"	1152.5 1/11

(3) 2x6'5	(1) 11 1/0 300	J 1/2	1133.3
	(I) 14 23 <i>0/</i> 360	2 5/16"	ITS2.3
	(1) 14 560	3 1/2"	IT53.5
	(2) 9-1/2 110	3 1/2"	MIT49
	(2) 9-1/2 230	4 5/8"	MIT35
	(2) 11-7/8 110	3 1/2"	MIT4II
5-1/4x5-1/4 PSL POST		· ·	MIT35
			HB7.12
BEAM SCHEDULE			
	(2) 14 230/360	4 5/8"	MIT35
I-3/4x9-I/2 LVL	(2) 14 560	7"	HB7.I2
3-1/2x9-1/2 PSL	* REQUIRES WEB 5	TIFFENERS	
5-I/4x9-I/2 PSL	FLIT	CH BEA	Μ
7x9-1/2 PSL	· • · · · · · · · · · · · · · · · · · ·		., ,
	3-1/2x9-1/2 PSL 5-1/4x9-1/2 PSL	(3) 2x6'5 4x4 6x6 3-1/2x3-1/2 PSL POST 3-1/2x5-1/4 PSL POST 5-1/4x5-1/4 PSL POST (1) 14 230/360 (2) 9-1/2 IIO (2) 9-1/2 230 (2) 11-7/8 IIO (2) 11-7/8 230/360 (2) 11-7/8 560 (2) 11-7/8 560 (2) 14 230/360 (2) 14 230/360 (2) 14 560 * REQUIRES WEB 51 5-1/4x9-1/2 PSL FLITC	(3) 2x6'5 4x4 6x6 3-1/2x3-1/2 PSL POST 3-1/2x5-1/4 PSL POST 5-1/4x5-1/4 PSL POST (2) 9-1/2 IIO (2) 9-1/2 IIO (3) 1/2" (2) 9-1/2 230 (3) 1/2" (2) 11-7/8 IIO (3) 1/2" (2) 11-7/8 1IO (3) 1/2" (2) 11-7/8 560 (3) 1/2" (2) 11-7/8 560 (3) 1/2" (4) 11-7/8 560 (5) 11-7/8 560 (7) 11-7/8 560 (7) 12-7/8 560 (7) 13-7/8 560 (7) 14-7/8 560 (7) 15-7/8 560 (7) 15-7/8 560 (7) 17-7/8 560 (7) 14-7/8 560 (7) 15-7/8 560 (8) 15-7/8 560 (9) 11-7/8 560 (1) 14-230/360 (2) 11-7/8 1IO (3) 1/2" (4) 11-7/8 1IO (5) 11-7/8 560 (7) 11-7/8 560 (7) 11-7/8 560 (7) 11-7/8 560 (9) 11-7/8 560 (1) 14-230/360 (1) 14-230/360 (2) 11-7/8 1IO (2) 11-7/8 560 (3) 1/2" (4) 15-7/8 1IO (5) 11-7/8 560 (7) 11-7/8 560

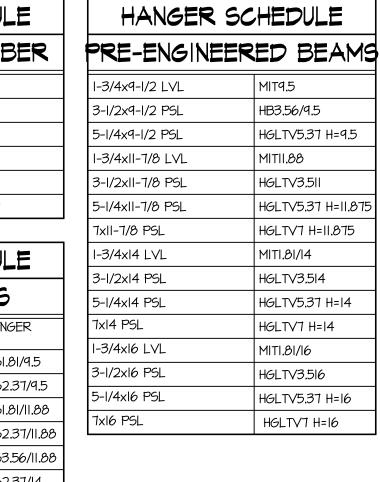
* REQUIRES WEB STIFFENERS				
		FLIT	CH BEA	M
	FB-I	I"x9" STEI w/ (2) I/2	EL P2 w/ (2) 1-3 " Φ THRU BOLT	3/4x9 S AT

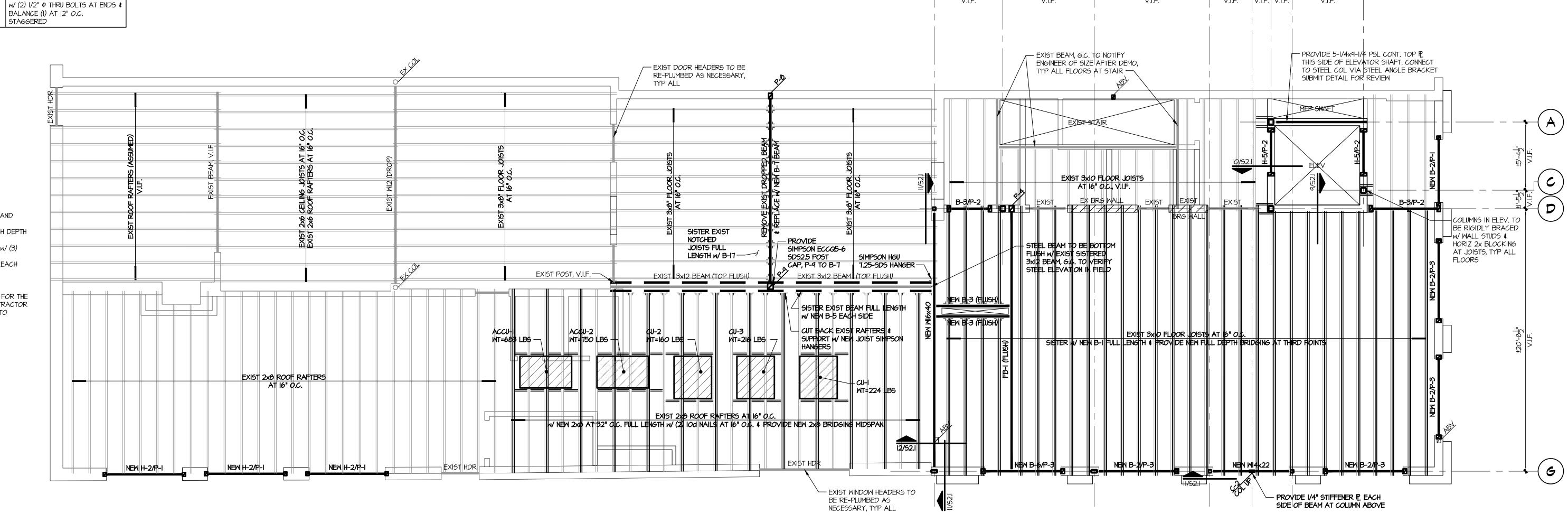
B-5 | I-3/4xII-7/8 LVL B-6 3-1/2x11-7/8 PSL B-7 | 5-1/4x11-7/8 PSL B-8 7xII-7/8 PSL B-9 | I-3/4xI4 LVL B-10 3-1/2x14 PSL B-II 5-1/4x14 PSL B-12 | 7x14 PSL B-I3 | I-3/4xI6 LVL B-14 3-1/2x16 PSL B-15 5-1/4x16 PSL B-16 | 7x16 PSL B-17 | 1-3/4x7-1/4 PSL B-18 5-1/4x18 PSL B-19 5-1/4x20 PSL

SEE STRUCTURAL NOTES FOR REQUIRED WOOD SPECIES AND PROVIDE 1/2" PLYWOOD SHIM BETWEEN EACH PLY. MATCH DEPTH

- OF HEADER FOR 2x8 MULTIPLE MEMBERS GLUE AND NAIL EACH PLY w/ (3) ROWS OF 16d NAILS AT 8" O.C.
- FOR 2x10 AND 2x12 MULTIPLE MEMBERS GLUE AND NAIL EACH PLY w/ (4) ROWS OF 16d NAILS AT 8" O.C. NAIL OR BOLT MULTIPLE LVL BEAMS AND HEADERS PER
- MANUFACTURERS REQUIREMENTS. PRE-ENGINEERED WOOD HEADERS MAY BE SUBSTITUTED FOR THE 2x WOOD HEADERS SHOWN ON THE DRAWINGS. THE CONTRACTOR SHALL SUBMIT SHOP DRAWINGS FOR APPROVAL PRIOR TO

CONSTRUCTION.







SECOND FLOOR FRAMING PLAN

I. THE CONTRACTOR SHALL VERIFY ALL EXISTING BUILDING INFORMATION SHOWN (DIMENSIONS, ELEVATIONS, ETC) AND NOTIFY THE ARCHITECT/ENGINEER OF ANY DISCREPANCIES PRIOR TO FABRICATION OF ANY STRUCTURAL COMPONENT. FAILURE TO NOTIFY ARCHITECT/ENGINEER OF UNSATISFACTORY CONDITIONS CONSTITUTES

- ACCEPTANCE OF UNSATISFACTORY CONDITIONS. 2. G.C. IS RESPONSIBLE TO FIELD VERIFY EXIST DIMENSIONS AND MEMBER SIZE. CONTACT ENGINEER IMMEDIATELY WITH ANY SIGNIFICANT DIFFERENCES TYPICAL AT ALL FLOORS.
- 3. PROVIDE TEMPORARY SHORING WHERE REQ'D TO MAINTAIN STABILITY. 4. TYPICAL FLOOR CONSTRUCTION 3/4" PLYWOOD SHEATHING ON WOOD FRAMED
- STRUCTURE. 5. ALL HEADERS TO BE DROPPED UNLESS NOTED OTHERWISE.
- 6. ALL 2x STRUCTURAL FRAMING TO BE #2 DOUG FIR OR BETTER. 7. ALL JOISTS SHALL HAVE ONE ROW OF BRIDGING AT THE MIDSPAN.
- 8. ALL RIM JOIST SHALL BE I-1/4" LSL RIM JOIST OR EQUAL TO SUPPORT A MINIMUM OF 3400 LB/FT TYPICAL ALL FLOORS.
- 9. ALL HEADERS TO BE MIN. (2) 2xIO'S IN 2x4 EXTERIOR WALL AND (3) 2xIO'S IN 2X6 EXTERIOR WALL UNLESS NOTED OTHERWISE ON PLAN. 10. ALL OPENINGS IN THE EXTERIOR WALL 6'-O" AND GREATER SHALL HAVE A DOUBLE JACK STUD AND KING STUD.
- II. REFER TO ARCH SECTIONS AND ELEVATIONS FOR PLATE HEIGHTS.
- 12. FOR TYPICAL DETAILS SEE DRAWING S4.1
- 13. FOR GENERAL NOTES SEE DRAWING SO.I 14. GENERAL CONTRACTOR IS RESPONSIBLE TO INSTALL ALL PROPRIETARY FLOOR FRAMING IN ACCORDANCE WITH ALL MANUFACTURER'S REQUIREMENTS. TYPICAL FOR
- ALL FLOOR AND ROOF FRAMING THAT IS PRE-ENGINEERED LUMBER. 15. INSTALL PROPER JOIST HANGERS AT ALL JOIST MEMBERS. THE INSTALLATION OF THE JOIST HANGERS SHALL COMPLY WITH THE MANUFACTURER'S RECOMMENDED DETAILS. 16. PLACE DOUBLE JOISTS UNDER WALLS ABOVE OR PROVIDE SOLID BLOCKING AT 24"
 - O.C. UNDER WALLS ABOVE WHERE NOT ALREADY INSTALLED. 17. H-X/P-X DENOTES HEADER/BEAM AND END POST DESIGNATION, RE: SCHEDULE FOR SIZE. POSTS SIZES INDICATED ON PLAN AT POST LOCATIONS ARE SPECIFIC TO HEADER/BEAM END ONLY.

Norwescap Old Sullivan Building Remodeling Project

PROJECT LOCATION

LOT:

BLOCK:

371 S Main Street Phillipsburg, NJ 08865 Warren County

350 Marshall Street Phillipsburg, NJ 08865



PROFESSIONAL ENGINEERS

ADDRESS P.O. BOX 628 FARMINGDALE, NJ 07727

732.938.2666 732.938.2661

CERTIFICATE OF AUTHORIZATION NO. KSI PROJECT #

Issues and Revisions No. Date Issues and Revisions 05/17/24 | ISSUE FOR BID

NJ License No: AI 15794 Benjamin J. Horten COND FLOOR FRAMING PLAN

\\ksi-far-dc-002\\data\\2023 Projects\\2384_Ben Horten\2384_02_371 Main St., Phillipsburg_Structural Drawings\2384_02_Plans_C.dwg

		1
+	HEADER SCHEDULE	HAN
H-I	(2) 2x8'5	DIME
H-2	(2) 2xIO'5	2.0
H-3	(2) 2xl2'5	2x8
H-4	(3) 2×8'5	(2) 2x8'S
H-5	(3) 2xIO'S	2xIO
H-6	(3) 2x 2'5	(2) 2x10'S
П-0	(3) 2812 3	2xl2

HANGER SCHEDULE		
DIMENSIONA	L LUMBER	
2x8	LUS26	
(2) 2x8'5	LUS26-2	
2xl0	LU528	
(2) 2x10'5	LUS28-2	
2xl2	LU5210	
(2) 2x12'5	LUS210-2	

C	DLUMN SCHEDULE
C-I	H55 3x3x3/8
C-2	HSS 6x3x3/8

C-3	HSS 5x5xI/4
F	POST SCHEDULE
P-I	(2) 2x4'5
P-2	(2) 2x6'5
P-3	(3) 2x4'5
P-4	(3) 2x6'5
P-5	4x4
P-6	6x6
P-7	3-1/2x3-1/2 PSL POST
P-8	3-I/2x5-I/4 PSL POST
P-9	5-I/4x5-I/4 PSL POST

P-8	3-1/2x5-1/4 PSL POST
P-9	5-1/4x5-1/4 PSL POST
E	BEAM SCHEDULE
B-I	I-3/4x9-I/2 LVL
B-2	3-1/2x9-1/2 PSL
B-3	5-1/4x9-1/2 PSL
B-4	7x9-1/2 PSL
B-5	I-3/4xII-7/8 LVL
B-6	3-1/2x11-7/8 PSL

	P-9	5-I/4x5-I/4 PSL POST	
_			
	E	BEAM SCHEDULE	
	B-I	I-3/4x9-I/2 LVL	
	B-2	3-1/2x9-1/2 PSL	
	B-3	5-1/4x9-1/2 PSL	
	B-4	7x9-1/2 PSL	
	B-5	I-3/4xII-7/8 LVL	
	B-6	3-1/2x11-7/8 PSL	
	B-7	5-1/4x11-7/8 PSL	
	B-8	TxII-7/8 PSL	
	B-9	I-3/4xI4 LVL	
	B-10	3-1/2x14 PSL	
	B-II	5-1/4x14 PSL	
	B-I2	TxI4 PSL	

B-13 | 1-3/4x16 LVL

B-14 3-1/2x16 PSL

B-15 5-1/4x16 PSL

B-17 | 1-3/4x7-1/4 PSL

B-18 5-1/4x18 PSL

B-19 5-1/4x20 PSL

B-16 7x16 PSL

DIMENSIC	NAL LU	MBER		KE-ENGINEE'
:8	LU526	;)	1-3	3/4x9-1/2 LVL
) 2x8'5	LUS26	5-2	3-1	1/2x9-1/2 PSL
:10	LUS28	3	5-1	/4x9-1/2 PSL
) 2x10'5	LUS28	3-2	1-3	3/4xII-7/8 LVL
12	LUS2I	0	3-1	/2x11-7/8 PSL
) 2xl2'5	LUS2I	<i>0</i> -2	5-1	/4x11-7/8 PSL
			٦×	II-7/8 PSL
HANGER	R SCHEI	OULE	1-3	3/4×14 LVL
			3-1	/2x14 PSL
MOOD		oT5	5-1	/4xI4 PSL
PIST	TTL FLANGE	HANGER	٦x١	4 PSL
9-I/2 II <i>O</i>	MIDTH I 3/4"	ITSI.81/9.5	1-3	/4xl6 LVL
-1-1/2 IIU	1 2/4	1101.01/1.0		(0.14. DS)

MOOD I-JOISTS				
TTL FLANGE WIDTH	HANGER			
I 3/4"	ITSI.8I/9.5			
2 5/16"	ITS2.37/9.5			
1 3/4"	ITSI.8I/II.88			
2 5/16"	ITS2.37/II.88			
3 1/2"	ITS3.56/II.88			
2 5/16"	ITS2.37/I4			
3 1/2"	ITS3.56/14			
3 1/2"	MIT49.5			
4 5/8"	MIT359.5-2			
3 1/2"	MIT411.88			
4 5/8"	MIT35II.88-2			
7"	HB7.12/11.88*			
4 5/8"	MIT3514-2			
	TTL FLANGE WIDTH 1 3/4" 2 5/16" 1 3/4" 2 5/16" 3 1/2" 2 5/16" 3 1/2" 4 5/8" 3 1/2" 4 5/8" 7"			

(2) 14 560	1"
* REQUIRES WEB ST	TIFFENERS

FLITCH BEAM FB-I I"x9" STEEL P w/ (2) I-3/4x9-I/4 LVL w/ (2) I/2" \$\phi\$ THRU BOLTS AT ENDS \$\psi\$ BALANCE (I) AT I2" O.C.

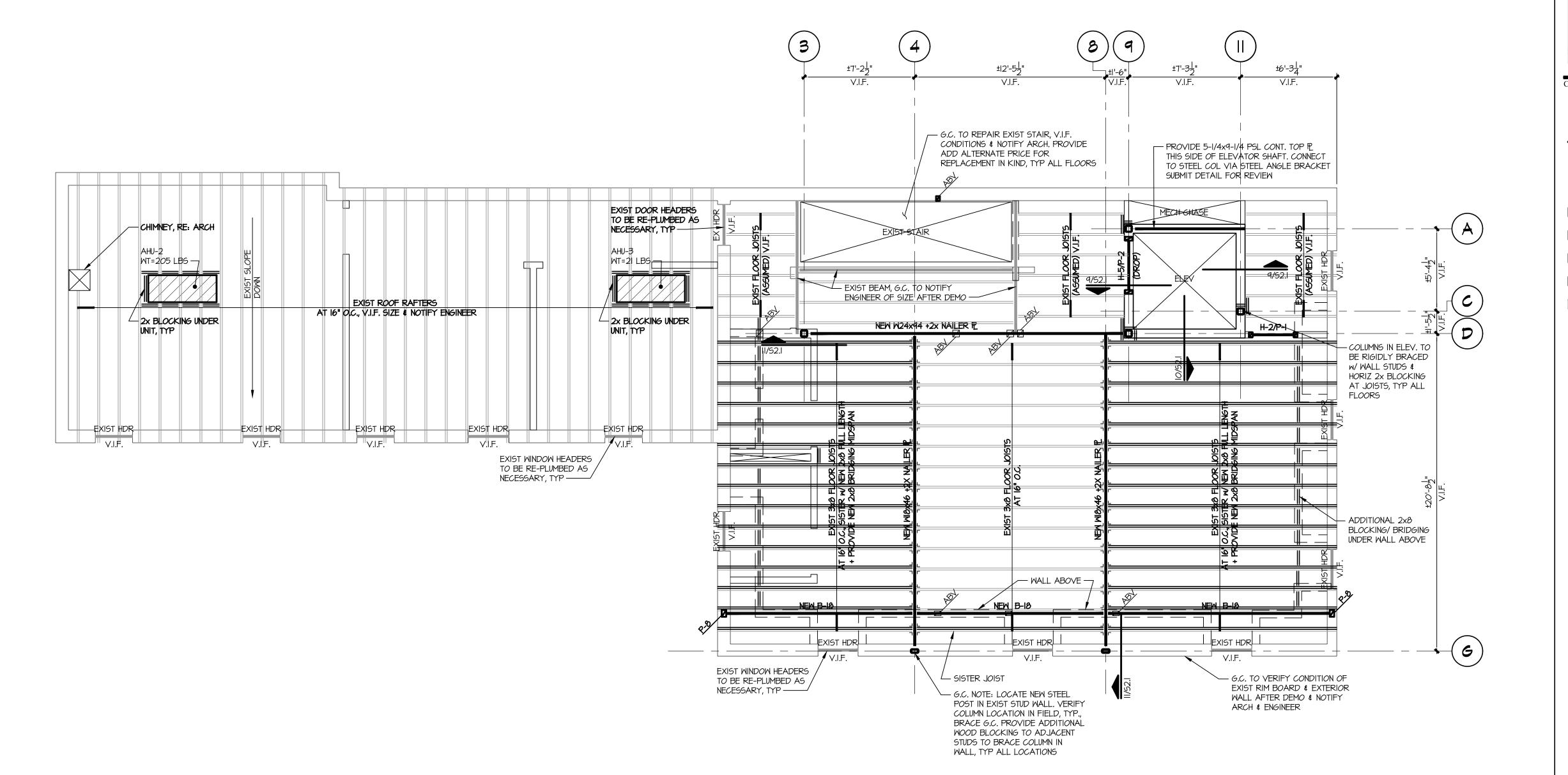
HB7.12/14*

SEE STRUCTURAL NOTES FOR REQUIRED WOOD SPECIES AND

- PROVIDE I/2" PLYWOOD SHIM BETWEEN EACH PLY. MATCH DEPTH OF HEADER
- FOR 2x8 MULTIPLE MEMBERS GLUE AND NAIL EACH PLY W/ (3) ROWS OF 16d NAILS AT 8" O.C.
- FOR 2x10 AND 2x12 MULTIPLE MEMBERS GLUE AND NAIL EACH PLY w/ (4) ROWS OF 16d NAILS AT 8" O.C.
- NAIL OR BOLT MULTIPLE LVL BEAMS AND HEADERS PER
- MANUFACTURERS REQUIREMENTS.
- PRE-ENGINEERED WOOD HEADERS MAY BE SUBSTITUTED FOR THE 2x WOOD HEADERS SHOWN ON THE DRAWINGS. THE CONTRACTOR SHALL SUBMIT SHOP DRAWINGS FOR APPROVAL PRIOR TO CONSTRUCTION.

HB3.56/9.5 HGLTV5.37 H=9.5 MITII.88 HGLTV3.511 HGLTV5.37 H=II.875 HGLTV7 H=11.875 MITI.81/14 HGLTV3.514 HGLTV5.37 H=14 HGLTV7 H=14 MITI.81/16 3-1/2x16 PSL HGLTV3.516 5-1/4x16 PSL HGLTV5.37 H=16 7x16 PSL HGLTV7 H=16

HANGER SCHEDULE





THIRD FLOOR FRAMING PLAN

I. THE CONTRACTOR SHALL VERIFY ALL EXISTING BUILDING INFORMATION SHOWN (DIMENSIONS, ELEVATIONS, ETC) AND NOTIFY THE ARCHITECT/ENGINEER OF ANY DISCREPANCIES PRIOR TO FABRICATION OF ANY STRUCTURAL COMPONENT. FAILURE TO NOTIFY ARCHITECT/ENGINEER OF UNSATISFACTORY CONDITIONS CONSTITUTES

- ACCEPTANCE OF UNSATISFACTORY CONDITIONS. 2. G.C. IS RESPONSIBLE TO FIELD VERIFY EXIST DIMENSIONS AND MEMBER SIZE. CONTACT ENGINEER IMMEDIATELY WITH ANY SIGNIFICANT DIFFERENCES TYPICAL AT ALL FLOORS.
- 3. PROVIDE TEMPORARY SHORING WHERE REQ'D TO MAINTAIN STABILITY. 4. TYPICAL FLOOR CONSTRUCTION 3/4" PLYWOOD SHEATHING ON WOOD FRAMED
- STRUCTURE. 5. ALL HEADERS TO BE DROPPED UNLESS NOTED OTHERWISE.
- 6. ALL 2x STRUCTURAL FRAMING TO BE #2 DOUG FIR OR BETTER. 7. ALL JOISTS SHALL HAVE ONE ROW OF BRIDGING AT THE MIDSPAN. 8. ALL HEADERS TO BE MIN. (2) 2xIO'S IN 2x4 EXTERIOR WALL AND (3) 2xIO'S IN 2X6
- EXTERIOR WALL UNLESS NOTED OTHERWISE ON PLAN. 9. ALL OPENINGS IN THE EXTERIOR WALL 6'-O" AND GREATER SHALL HAVE A DOUBLE JACK STUD AND KING STUD.
- IO. REFER TO ARCH SECTIONS AND ELEVATIONS FOR PLATE HEIGHTS. II. FOR TYPICAL DETAILS SEE DRAWING S4.I
- 12. FOR GENERAL NOTES SEE DRAWING SO.I
- 13. GENERAL CONTRACTOR IS RESPONSIBLE TO INSTALL ALL PROPRIETARY FLOOR FRAMING IN ACCORDANCE WITH ALL MANUFACTURER'S REQUIREMENTS. TYPICAL FOR ALL FLOOR AND ROOF FRAMING THAT IS PRE-ENGINEERED LUMBER. 14. INSTALL PROPER JOIST HANGERS AT ALL JOIST MEMBERS. THE INSTALLATION OF THE JOIST HANGERS SHALL COMPLY WITH THE MANUFACTURER'S RECOMMENDED DETAILS. 15. PLACE DOUBLE JOISTS UNDER WALLS ABOVE OR PROVIDE SOLID BLOCKING AT 24"
- O.C. UNDER WALLS ABOVE WHERE NOT ALREADY INSTALLED. 16. H-X/P-X DENOTES HEADER/BEAM AND END POST DESIGNATION, RE: SCHEDULE FOR SIZE. POSTS SIZES INDICATED ON PLAN AT POST LOCATIONS ARE SPECIFIC TO HEADER/BEAM END ONLY.

Norwescap Old Sullivan Building Remodeling Project

PROJECT LOCATION

LOT:

BLOCK:

371 S Main Street Phillipsburg, NJ 08865 Warren County

350 Marshall Street Phillipsburg, NJ 08865



PROFESSIONAL ENGINEERS

ADDRESS P.O. BOX 628 FARMINGDALE, NJ 07727 732.938.2666

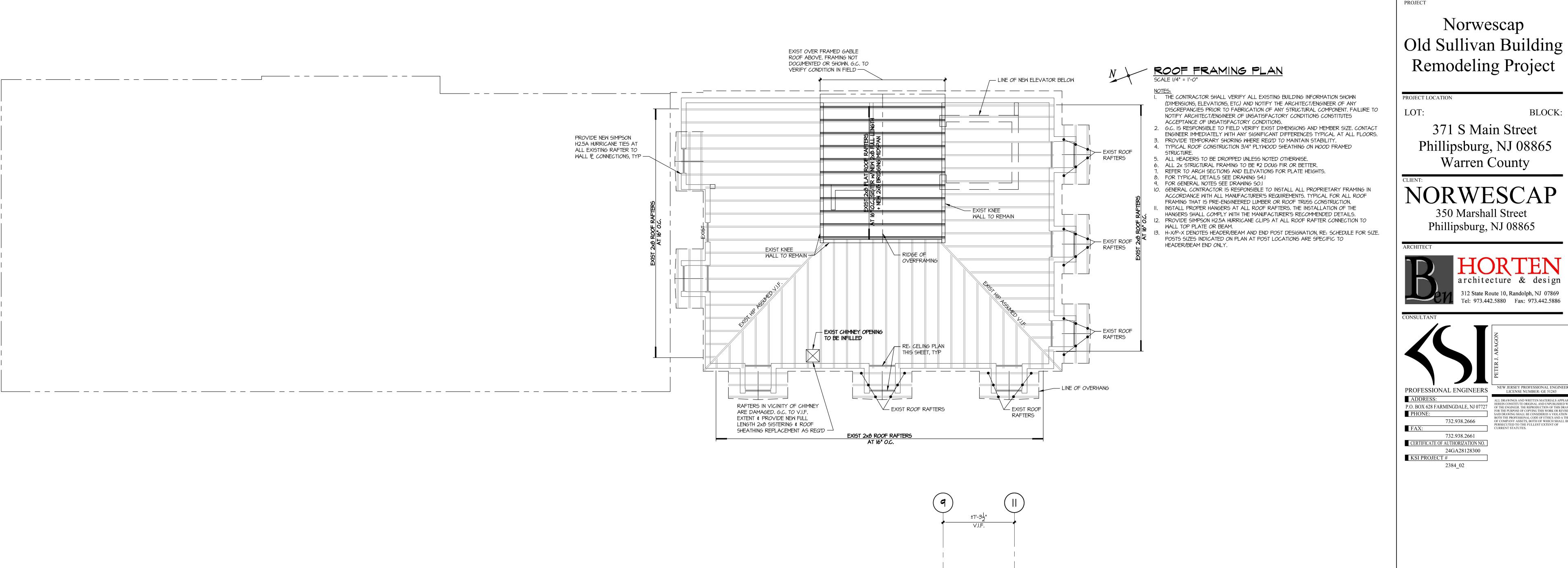
732.938.2661

CERTIFICATE OF AUTHORIZATION NO. KSI PROJECT #

Issues and Revisions No. Date Issues and Revisions 1. 05/17/24 ISSUE FOR BID

NJ License No: AI 15794 Benjamin J. Horten THIRD FLOOR FRAMING PLAN

\\ksi-far-dc-002\data\2023 Projects\2384_Ben Horten\2384_02_371 Main St., Phillipsburg_Structural Drawings\2384_02_Plans_C.dwg



BLOCKING AT 24"

O.C. STAGGERED

_ EX HDR

350 Marshall Street

371 S Main Street

Warren County

Norwescap

BLOCK:

Phillipsburg, NJ 08865



architecture & design 312 State Route 10, Randolph, NJ 07869 Tel: 973.442.5880 Fax: 973.442.5886



P.O. BOX 628 FARMINGDALE, NJ 07727

732.938.2666

732.938.2661 CERTIFICATE OF AUTHORIZATION NO. 24GA28128300

- G.C. TO COORD ELEVATOR CEILING FRAMING REQUIREMENTS

ELEVATION W/

ELEVATOR MFR-

W8xI8 HOIST BEAM

2x8 FULL LENGTH

NEW 2x8 BLOCKING

UNDER EX BRG WALL

ABV. G.C. TO VERIFY

NEW SIMPSON HANGER

EX HDR

SIZE TO BE V.I.F. BY

G.C. FOR ALL EXIST

JOISTS/BEAM, TYP.

- G.C. TO COORD ATTACHMENT

W/ FOLDING PARTITION MFR

REQ'MENTS OF FOLDING PARTITION

EXIST HEADERS TO BE RE-PLUMBED

AS NECESSARY, TYP ALL ----

WALL ALIGNMENT

BLOCKING

AHU-I

EX HDR

- INFILL AT EXIST CHIMNEY

OPENING AS REQ'D

— LINE OF WALL ABOVE

WT=230 LBS-

BETWEEN JOISTS UNDER UNIT

- I. THE CONTRACTOR SHALL VERIFY ALL EXISTING BUILDING INFORMATION SHOWN (DIMENSIONS, ELEVATIONS, ETC) AND NOTIFY THE ARCHITECT/ENGINEER OF ANY DISCREPANCIES PRIOR TO FABRICATION OF ANY STRUCTURAL COMPONENT. FAILURE TO NOTIFY ARCHITECT/ENGINEER OF UNSATISFACTORY CONDITIONS CONSTITUTES
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- 4. TYPICAL FLOOR CONSTRUCTION 3/4" PLYWOOD SHEATHING ON WOOD FRAMED STRUCTURE. 5. ALL HEADERS TO BE DROPPED UNLESS NOTED OTHERWISE.
- 6. ALL 2x STRUCTURAL FRAMING TO BE #2 DOUG FIR OR BETTER.
- 7. ALL JOISTS SHALL HAVE ONE ROW OF BRIDGING AT THE MIDSPAN. 8. ALL HEADERS TO BE MIN. (2) 2xIO'S IN 2x4 EXTERIOR WALL AND (3) 2xIO'S IN 2X6
- EXTERIOR WALL UNLESS NOTED OTHERWISE ON PLAN. 9. ALL OPENINGS IN THE EXTERIOR WALL 6'-O" AND GREATER SHALL HAVE A DOUBLE JACK STUD AND KING STUD.
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- II. FOR TYPICAL DETAILS SEE DRAWING S4.1 12. FOR GENERAL NOTES SEE DRAWING SO.I
- 13. GENERAL CONTRACTOR IS RESPONSIBLE TO INSTALL ALL PROPRIETARY FLOOR FRAMING IN ACCORDANCE WITH ALL MANUFACTURER'S REQUIREMENTS. TYPICAL FOR ALL FLOOR AND ROOF FRAMING THAT IS PRE-ENGINEERED LUMBER.
- 14. INSTALL PROPER JOIST HANGERS AT ALL JOIST MEMBERS. THE INSTALLATION OF THE JOIST HANGERS SHALL COMPLY WITH THE MANUFACTURER'S RECOMMENDED DETAILS. 15. PLACE DOUBLE JOISTS UNDER WALLS ABOVE OR PROVIDE SOLID BLOCKING AT 24"
- O.C. UNDER WALLS ABOVE WHERE NOT ALREADY INSTALLED. 16. H-X/P-X DENOTES HEADER/BEAM AND END POST DESIGNATION, RE: SCHEDULE FOR SIZE. POSTS SIZES INDICATED ON PLAN AT POST LOCATIONS ARE SPECIFIC TO HEADER/BEAM END ONLY.

HE	ADER SCHEDULE
H-I	(2) 2x8'5
H-2	(2) 2xIO'5
H-3	(2) 2xl2'5
H-4	(3) 2x8'5
H-5	(3) 2x10'5
H-6	(3) 2xl2'5

H-5	(5) 2XIU 5
H-6	(3) 2xl2'5
L	POST SCHEDULE
P-I	(2) 2x4'5
P-2	(2) 2x6'5
P-3	(3) 2x4'5
P-4	(3) 2x6'5
P-5	4x4
P-6	6x6
P-7	3-1/2x3-1/2 PSL POST
P-8	3-1/2x5-1/4 PSL POST
P-9	5-1/4x5-1/4 PSL POST

B-2	3-1/2x9-1/2 PSL			
B-3	5-I/4x9-I/2 PSL			
B-4	7x9-1/2 PSL			
B-5	I-3/4xII-7/8 LVL		<u> </u>	ANGER S
B-6	3-1/2x11-7/8 PSL		Π/-	
B-7	5-1/4xII-7/8 PSL		DIM	Ensiona
B-8	7xII-7/8 PSL		2x8	
B-9	I-3/4xI4 LVL		(2) 2x8'S	
B-10	3-1/2x14 PSL		2xIO	
B-II	5-I/4xI4 PSL		(2) 2x10'5	 3
B-I2	7xI4 PSL		2xl2	
B-13	I-3/4xI6 LVL		(2) 2x12 ¹	
B-14	3-1/2x16 PSL		(2/ 2X12 3	,
B-15	5-1/4x16 PSL	١		
B-16	7x16 PSL			olumn so
B-17	I-3/4x7-I/4 PSL		C-I	HSS 3x3x3/8
B-18	5-1/4x18 PSL		C-2	HSS 6x3x3/8

BEAM SCHEDULE

B-I | I-3/4x9-I/2 LVL

B-19 5-1/4×20 PSL

DIMENSION	AL LUMBER
2x8	LUS26
(2) 2x8'S	LUS26-2
2xIO	LU528
(2) 2x10'5	LUS28-2
2xl2	LU5210
(2) 2x12'5	LUS210-2
	•
COLUMN	SCHEDULE

C-3 HSS 5x5x1/4

PRE-ENGINEERED BEAMS				
I-3/4x9-I/2 LVL	MIT9.5			
3-1/2x9-1/2 PSL	HB3.56/9.5			
5-1/4x9-1/2 PSL	HGLTV5.37 H=9.5			
1-3/4x11-7/8 LVL	MITII.88			
3-1/2x11-7/8 PSL	HGLTV3.511			
5-1/4x11-7/8 PSL	HGLTV5.37 H=11.875			
7xII-7/8 PSL	HGLTV7 H=II.875			
1-3/4x14 LVL	MITI.81/14			
3-1/2x14 PSL	HGLTV3.514			
5-1/4x14 PSL	HGLTV5.37 H=14			
7xI4 PSL	HGLTV7 H=14			
1-3/4x16 LVL	MITI.81/16			
3-1/2x16 PSL	HGLTV3.516			
5-1/4x16 PSL	HGLTV5.37 H=16			

7x16 PSL

I-3/4x9-I/2 LVL	MIT9.5
3-1/2x9-1/2 PSL	HB3.56/9.5
5-1/4x9-1/2 PSL	HGLTV5.37 H=9.5
I-3/4xII-7/8 LVL	MITII.88
3-1/2x11-7/8 PSL	HGLTV3.511
5-1/4x11-7/8 PSL	HGLTV5.37 H=II.875
TxII-T/8 PSL	HGLTV7 H=II.875
I-3/4xI4 LVL	MITI.81/14
3-1/2x14 PSL	HGLTV3.514
5-1/4x14 PSL	HGLTV5.37 H=14
7x14 PSL	HGLTV7 H=14
I-3/4xI6 LVL	MITI.81/16
3-1/2x16 PSL	HGLTV3.516
5-1/4x16 PSL	HGI TV5 37 H=16

HGLTV7 H=16

HANGER SCHEDULE

I. SEE STRUCTURAL NOTES FOR REQUIRED WOOD SPECIES AND 2. PROVIDE I/2" PLYWOOD SHIM BETWEEN EACH PLY. MATCH DEPTH OF HEADER 3. FOR 2x8 MULTIPLE MEMBERS GLUE AND NAIL EACH PLY W/ (3) ROWS OF 16d NAILS AT 8" O.C. 4. FOR 2xIO AND 2xI2 MULTIPLE MEMBERS GLUE AND NAIL EACH

WHERE CEILING JOISTS LAP W/ ROOF RAFTERS, REINFORCE W/ (4) I/4" ϕ LAG SCREWS. PROVIDE ADD'L SHIMS AS REQ'D, TYP. —

DENOTES EXISTING STEEP SLOPED RAFTER

STUDS. G.C.TO V.I.F. EXIST CONDITION AND

SISTER ANY EXIST WATER DAMAGED

GENERAL CONTRACTOR TO PROVIDE SF

MANSARD RAFTERS RAFTER REPAIRS.

TYPICAL FOR PERIMETER OF MANSARD —

UNIT COST REPAIRS FOR ROOF AND WALL

SHEATHING AND UNIT COST FOR JOIST AND

MEMBERS AS REQ'D. PART OF BID,

PLY w/ (4) ROWS OF 16d NAILS AT 8" O.C. 5. NAIL OR BOLT MULTIPLE LVL BEAMS AND HEADERS PER MANUFACTURERS REQUIREMENTS. 6. PRE-ENGINEERED WOOD HEADERS MAY BE SUBSTITUTED FOR THE 2x WOOD HEADERS SHOWN ON THE DRAWINGS. THE CONTRACTOR SHALL SUBMIT SHOP DRAWINGS FOR APPROVAL PRIOR TO CONSTRUCTION.

110.	Daic	133003 0110 110 1310113	Dy	Officer
1.	05/17/24	ISSUE FOR BID	SA	ВН
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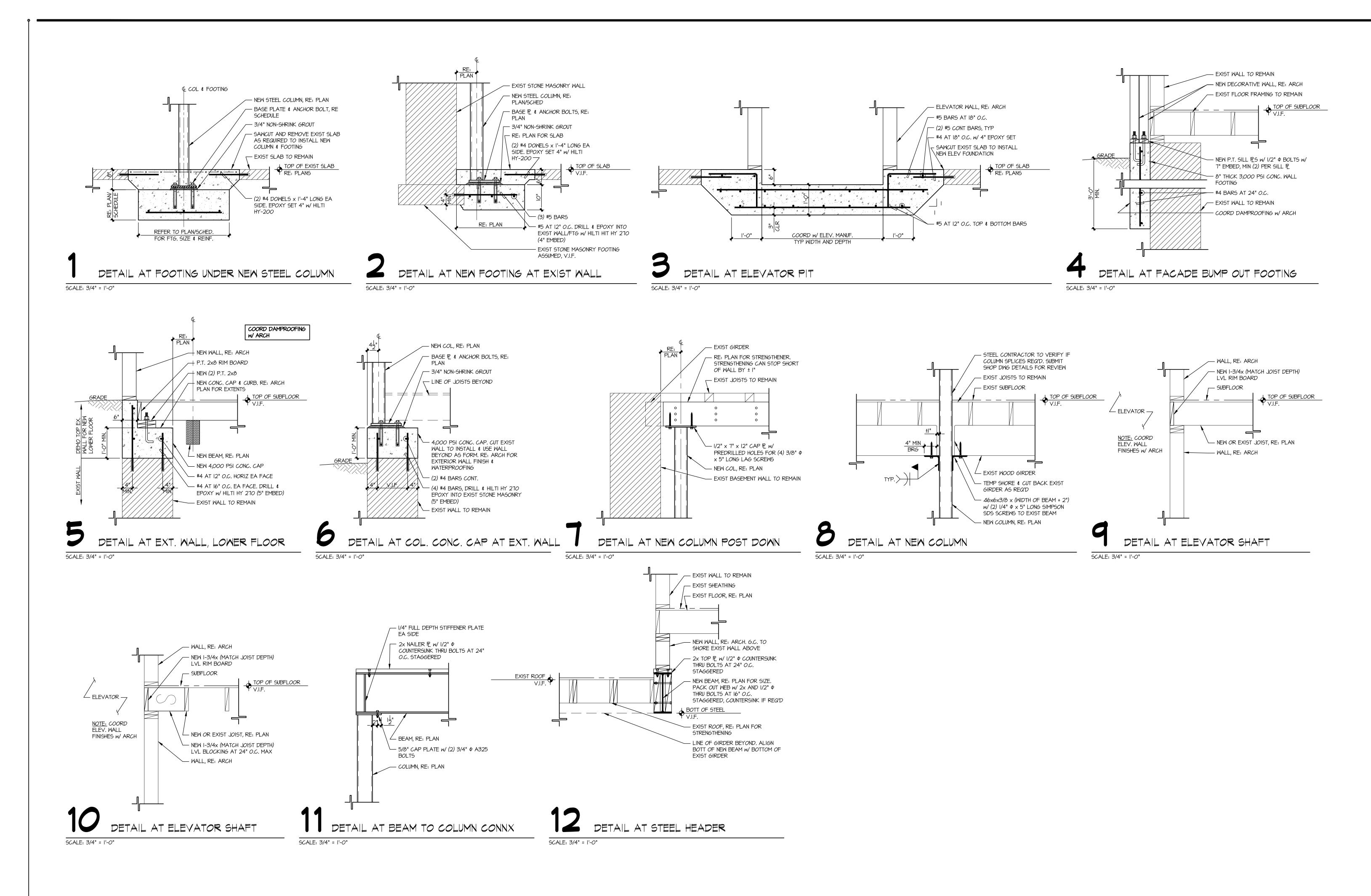
Registration and Signature

Issues and Revisions

No. Date Issues and Revisions

NJ License No: AI 15794 Benjamin J. Horten ROOF & CEILING FRAMING PLAN

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Norwescap Old Sullivan Building Remodeling Project

PROJECT LOCATION LOT:

BLOCK:

371 S Main Street Phillipsburg, NJ 08865 Warren County

350 Marshall Street Phillipsburg, NJ 08865



PROFESSIONAL ENGINEERS

ADDRESS P.O. BOX 628 FARMINGDALE, NJ 07727 732.938.2666

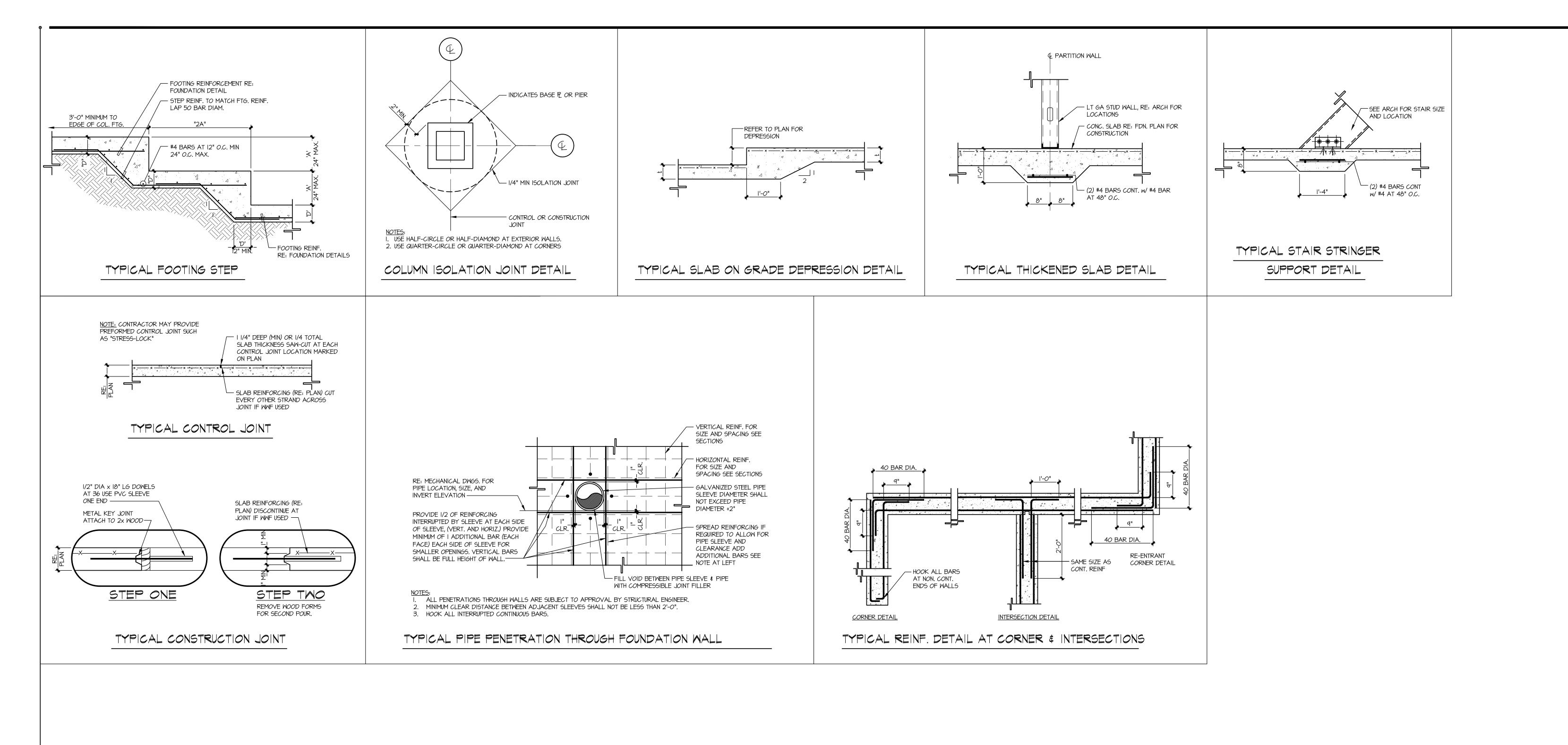
732.938.2661 CERTIFICATE OF AUTHORIZATION NO.

24GA28128300 KSI PROJECT #

Issues and Revisions No. Date Issues and Revisions 1. 05/17/24 ISSUE FOR BID

NJ License No: AI 15794

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Issues and Revisions

No. Date Issues and Revisions

1. 05/17/24 ISSUE FOR BID

SA BH

BH

Registration and Signature

Norwescap

Old Sullivan Building

Remodeling Project

371 S Main Street

Phillipsburg, NJ 08865

Warren County

350 Marshall Street

Phillipsburg, NJ 08865

312 State Route 10, Randolph, NJ 07869 Tel: 973.442.5880 Fax: 973.442.5886

BLOCK:

PROJECT LOCATION

LOT:

ADDRESS

P.O. BOX 628 FARMINGDALE, NJ 07727

CERTIFICATE OF AUTHORIZATION NO.

KSI PROJECT #

732.938.2666

732.938.2661

24GA28128300

Benjamin J. Horten NJ License No: AI 15794

Drawing Description:

TYPICAL DETAILS

Computer File:
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Horten\2384_02_371 Main St., Phillipsburg_Structural
Drawings\2384_02_Details_B.dwg

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